

- Infrastructure Design
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Proposed Artificial Grass Pitch Northfleet School for Girls Hall Road Northfleet Gravesend

Drainage Strategy

Revision A: July 2025
R-DS-28616-01-A

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Proposed Artificial Grass Pitch
Northfleet School for Girls
Hall Road
Northfleet
Gravesend

Drainage Strategy

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Revision	Date	Description	Prepared	Reviewed
0	30 th August 2024	First Issue	Josh Beverly	Katherine Rose
A	21 st July 2025	Revised following LLFA comments and additional soakaway testing.	Thomas Kirk	Katherine Rose

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1.0 Introduction

1.1 Background

1.1.1 This report is a Drainage Strategy which has been prepared by JPP Consulting Ltd for the benefit of our Client, Labosport Ltd (to be utilised in conjunction with the Football Foundation AGP Framework), for a proposed Artificial Grass Pitch (AGP).

1.1.2 The proposed artificial grass pitch is located at Northfleet School for Girls, Hall Road, Northfleet, as shown in Figure 1.1 below. Northfleet is located to the south-east of London, east of Dartford, south of the River Thames and west of Gravesend. The National Grid Reference for the site is E562960 N172690. The proposed development has a total development area of 0.643ha (6,425m²).

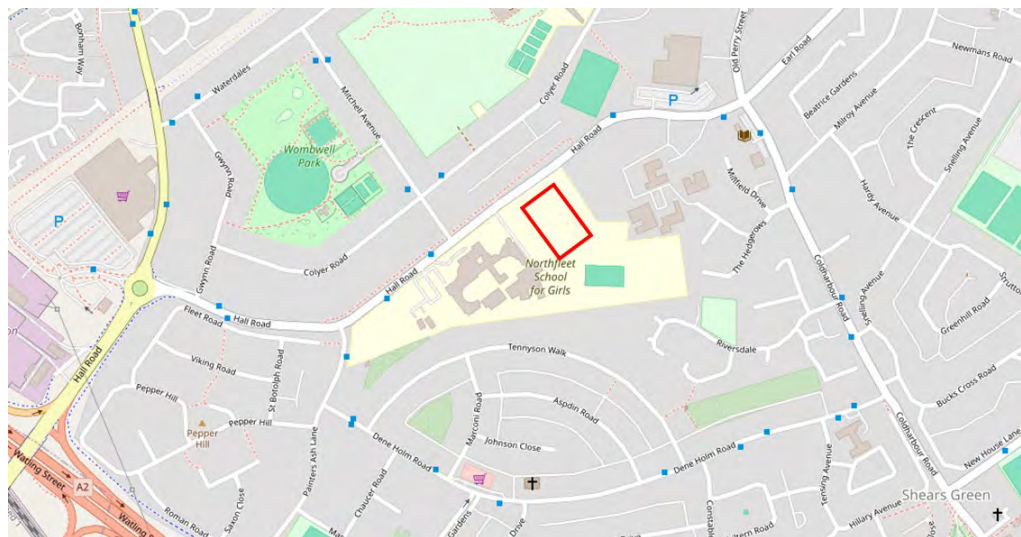


Figure 1.1 Site Location Plan

Source: OpenStreetMap

Obtained: 16/07/2024

1.2 Objectives

1.2.1 The objective of this report is to advise interested parties regarding the management of surface water run-off arising from the proposals.

1.2.2 This report has been prepared to support a detailed planning application.

1.3 Reference documents

1.3.1 This report has been prepared with reference to the following publications:-

- Ministry of Housing, Communities and Local Government (March 2012, updated December 2023), National Planning Policy Framework
- Ministry of Housing, Communities and Local Government (March 2014, updated August 2022), Planning Practice Guidance 'Flood Risk and Coastal Change'
- Department for Environment, Food and Rural Affairs (March 2015), Non-statutory technical standards for sustainable drainage systems
- Environment Agency (September 2013), Climate Change Allowances for Planners: Guidance to support the National Planning Policy Framework
- Environment Agency (October 2013), Delivering benefits through evidence: Rainfall runoff management for developments
- HM Government (2010), The Building Regulations (2010), Drainage and Waste Disposal, Approved Document H, The NBS, Newcastle Upon Tyne
- Wilson, Bray, Cooper (2004), Sustainable drainage systems: Hydraulic, structural and water quality advice, C609, CIRIA, London
- Woods-Ballard et al (2015), The SUDS Manual, C753, CIRIA, London
- CIRIA Report C624 Development and flood risk
- National SUDS Working Group (2004), Interim Code of Practice for Sustainable Drainage Systems,
- Institute of Hydrology (1999), Flood Estimation Handbook, Institute of Hydrology, Wallingford
- BS EN 752:2008 Drain and sewer systems outside buildings. Hydraulic design and environmental considerations
- BS 8533:2011 Assessing and managing flood risk in development – Code of Practice
- CIRIA Report C635 Designing for exceedance in urban drainage – good practice

1.4 Report history

- 1.4.1 This report has been revised (Revision A) in receipt of consultation comments from Kent County Council (KCC) as the Lead Local Flood Authority (LLFA) and BRE-365 soakage testing carried out by JPP Geotechnical & Environmental. Revision A includes the infiltration testing results and a revised surface water drainage layout.
- 1.4.2 Changes to the report associated with Revision A are indicated by a vertical line in the right-hand column.

2.0 Description and history of the site and development proposals

2.1 Location and description of the site

- 2.1.1 The proposed artificial grass pitch is located at Northfleet School for Girls, Hall Road, Northfleet, as shown in Figure 1.1 above. The site sits within the eastern part of the wider site for Northfleet School for Girls, with playing fields and courts to the south-east, and school buildings to the west. Hall Road is located to the north of the site, whilst trees are located to the east with Wombwell Hall Care Home beyond.

2.2 History of the site

- 2.2.1 The site is currently playing fields associated with the school, as shown on the topographical survey enclosed in **Appendix A**. We are not aware of any previous development on the site.
- 2.2.2 Aerial imagery dating back to December 2003 shows that the site was comparable at this time, see Figure 2.1 below.



Figure 2.1 Historic Aerial Imagery dated December 2003
Source: Google Earth Pro

- 2.2.3 The most recent aerial imagery of the site dated March 2024 is shown below in Figure 2.2.



Figure 2.2 Aerial Imagery dated March 2024
Source: Google Earth Pro

2.3 Proposed development

- 2.3.1 The proposed development will comprise an artificial grass pitch. The proposed development layout is shown on the plan enclosed in **Appendix B**.

2.4 Site topography

- 2.4.1 The topographical survey indicates that site levels within the footprint of the proposed AGP fall from the south corner (at approximately 33.0m) towards the north corner (lowest point of approximately 28.3m). The topographical survey is shown on the plan enclosed in **Appendix A**.

2.5 Existing drainage infrastructure

- 2.5.1 Southern Water's asset plan is enclosed in **Appendix C**. The asset plan identifies no adopted sewers within at the site location or across the wider school site. The nearest surface water is shown to the north-west at manhole reference MH9850 located in Hall Road. This is the head of the run and the sewer flows north away from the site.
- 2.5.2 The topographical survey, enclosed in **Appendix A**, identified no manholes within the footprint of the proposed pitch. A number manholes are shown along the western edge of the playing fields, understood to be existing drainage associated with the wider school site.

2.6 Geology of the site and ground investigation data

2.6.1 Inspection of the geological maps show that the upper-most bedrock geology which underlies the site is Thanet Formation – Sand. There are no superficial deposits recorded for the location of the site.

2.6.2 JPP Geotechnical & Environmental have completed a geotechnical investigation on the site with fieldwork carried out on the 9th February 2024, see report extract enclosed in **Appendix D**. This investigation found Topsoil overlying Made Ground which overlays Thanet Formation. Details of the encountered ground conditions are summarised in Table 2.1 below.

Summary of Ground Investigation Information							
Ground Conditions	Depth (m)						
	WS01	WS02	WS03	WS04	WS05	WS06	WS07
Topsoil	0.30	0.35	0.40	0.30	0.30	0.30	0.30
Made Ground	1.00	-	-	-	-	-	-
Thanet Formation	1.20	1.00	2.00	1.00	1.00	1.00	1.00
Source: JPP G&E ref. R-SI-27677-01-00							

Table 2.1 Ground Investigation Information

2.6.3 No groundwater was encountered during the investigation. The GI report notes:

‘...considered likely to be deep however there is always a potential for shallow perched groundwater to be present’.

2.6.4 The ground investigation highlights that there is a moderate risk of chalk dissolution within the pitch location. Design of the pitch, floodlight foundations and drainage should take into account the risk of chalk dissolution and encountering such features.

2.6.5 Infiltration testing has been carried out on-site (28th April 2025) in general accordance with BRE 365 ‘Soakaway Design’, comprising of four excavated trial pits. The observed soakaway testing results are summarised below in Table 2.2 and enclosed in **Appendix E**.

Summary of Infiltration Testing			
Trial Pit Position	Cycle	Base Depth of Trial Pit	Infiltration Rate
SA01	1	2.77m bgl	2.39 x 10 ⁻⁵ m/s
	2	2.85m bgl	1.24 x 10 ⁻⁵ m/s
	3	2.60m bgl	1.11 x 10 ⁻⁵ m/s (Conservatively extrapolated)
SA02	1	2.00m bgl	Insufficient infiltration to derive rate

SA03	1	1.60M bgl	4.50 x 10 ⁻⁶ m/s (Conservatively extrapolated)
SA04	1	2.45m bgl	5.28 x 10 ⁻⁶ m/s (Conservatively extrapolated)

Source: JPP G&E ref. 27677/LR/LC/L01

ToTable 2.2 Infiltration Testing Information

2.6.6 JPP G&E concludes the following with regards to the use of infiltration techniques:

'Based on the results of the infiltration testing, the near surface Thanet Formation deposits provide some infiltration potential which improves with depth...It is therefore recommended that the proposed soakaway has a base depth of at least 3.0m bgl, into the light yellowish brown slightly clayey slightly gravelly silty fine Sand, and the conservative lowest rate is adopted for design of 1.11 x 10⁻⁵ m/s. The trial pit logs and infiltration test results, should be referred to for full details of the ground conditions encountered at the depths of the testing to target for the soakaway design.'

2.6.7 With regards to chalk dissolution risk, the latest information presented by JPP G&E states:

'We would still suggest that a cautious approach is maintained of locating the soakaway at least 10m from foundations, structures (including the pitch) and boundaries, based on CIRIA 574 'Engineering in Chalk', where the chalk is of low density (assumed to be the case for the site in the absence of encountering the Chalk deposits within the excavations to allow testing to confirm otherwise).

For the soakaway design, as large and level area as possible is recommended to increase the surface area and targeting the most permeable deposits along its length, with ground conditions varying in fines and granular content and to minimise the potential for concentrated ingress occurring. A crate/geocellular type system as opposed to gravel is recommended to minimise potential for loss into a dissolution feature. It is recommended that drains should include sealed flexible joints or with concrete bedding and haunching to minimise the risk of future damage.

3.0 Management of surface water

3.1 Current conditions

- 3.1.1 The site is currently open with no existing development, and is therefore considered a greenfield site with no existing drainage. Therefore, greenfield run-off calculations shall be used to derive the allowable run off rate from the site.

3.2 Surface water drainage outfalls

- 3.2.1 It is a requirement of The Building Regulations (2010), Drainage and Waste Disposal, Approved Document H, to dispose of surface water collected by a development in accordance with the following, listed in order of priority:-

1. Infiltration systems where ground condition permit
2. To watercourses
3. To sewers

- 3.2.2 Each of these is considered separately below:

3.2.3 Infiltration systems

- 3.2.3.1 The geology of the site is described in 2.6 above. Infiltration test results show that soakaways are a viable method for managing surface water at the site,
- 3.2.3.2 As such, surface water will discharge the site via infiltration techniques, taking into account the risks identified associated with the chalk dissolution.

3.2.4 Watercourses / Main River

- 3.2.4.1 There are no watercourses located within or adjacent to the boundary of the proposed development.
- 3.2.4.2 Draining the site via infiltration techniques is a viable option, therefore discharging surface water via a watercourse will not be considered further.

3.2.5 Sewers

- 3.2.5.1 Southern Water's asset plan is enclosed in **Appendix C**. The asset plan identifies no adopted sewers within the development area nor the wider school site.
- 3.2.5.2 Draining the site via infiltration techniques is a viable option therefore the use of surface water sewers will not be considered further.

3.3 Surface water drainage strategy

- 3.3.1 Surface water from the development will drain via infiltration techniques. Infiltration will be facilitated via a geocellular soakaway, located to the north-west corner of the proposed AGP. The permeable sub-base of the pitch will both provide a level of attenuation and treatment of surface water. To benefit from the capacity available within the permeable sub-base of the pitch, surface water flows from the pitch will be restricted prior to connecting into the soakaway.
- 3.3.2 With reference to the risk of chalk dissolution set out within the Ground Investigation report, the pitch and the soakaway will both be appropriately lined and sealed.
- 3.3.3 An indicative surface water layout plan is enclosed in **Appendix F**. The drawing shows that surface water attenuation can be accommodated on the site and the method of discharge on the basis that infiltration techniques are viable. The detailed design parameters of the drainage are described below.

3.4 SUDS assessment

- 3.4.1 We have considered the suitability of SUDS for use on the development site. The review is set out in below Table 3.1.

SUDS Assessment		
SUDS Technique	Suitability	Justification
Rain Water Harvesting	No	Not applicable for the development type.
Green Roofs	No	Not applicable for the development type.
Infiltration	Yes	Soakaways are to be used.
Filter Strips / Filter Drains	No	Not suitable for the development type.
Swales	No	Not suitable for the development type.
Bioretention Systems	No	No open spaces.
Trees	No	The development proposals simply comprise the proposed AGP – no new trees will be provided within the sub-base for the new pitch.
Pervious Pavements	Yes	Surface water attenuation will be provided within the sub-base of the AGP.
Attenuation Tanks	Yes	Utilised to facilitate infiltration.
Detention Basin	No	Attenuation will be provided within the sub-base of the AGP and geocellular soakaway.
Ponds and Wetlands	No	Attenuation will be provided within the sub-base of the AGP and geocellular soakaway.
Trapped Drainage	No	Water treatment will be provided through the permeable sub-base of the AGP.

Table 3.1 SUDS Assessment

3.5 Water quality

- 3.5.1 Chapter 26 of The SuDS Manual 2015 (CIRIA 753) provides guidance on the methods that should be used to design SuDS to meet the water quality design criteria and good practice design standards. Based on the simple index approach, the pollution hazard indices for different land use classifications are listed in Table 26.2, Chapter 26 of the SuDS Manual. Table 3.2 below summarises the pollution hazard indices that are applicable for residential land use development.

Pollution Hazard Indices for Different Land Use Classifications				
Land Use	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydro-carbons
Residential Roofs	Very low	0.2	0.2	0.05
Source: Table 26.2 of The SuDS Manual				

Table 3.2 Pollution Hazard Indices for Different Land Use Classifications

- 3.5.2 'Residential Roofs' is considered to be the most appropriate category for the proposed pitch, due to the minimal pollution hazard associated with the proposed 3G Artificial Turf Pitch, which will have no vehicle movements within its extent.

- 3.5.3 Table 26.4 of The SuDS Manual shows the mitigation indices for a range of SuDS components for discharges to groundwaters. An extract of this is provided in Table 3.3 below for the components relevant to the proposed development.

Indicative SuDS Mitigation Indices for Discharges to Groundwaters			
Characteristics of the material overlying the proposed infiltration surface, through which the run-off percolates	Mitigation Indices		
	TSS	Metals	Hydro-carbons
A soil with good contaminant attenuation potential of at least 300mm in depth	0.4	0.3	0.3
Source: Table 26.4 of The SuDS Manual			

Table 3.3 Indicative SuDS Mitigation Indices for Discharges to Groundwaters

- 3.5.4 Table 26.3 of The SuDS Manual shows the mitigation indices for a range of SuDS components for discharges to surface waters. An extract of this is provided in Table 3.4, representing the proposed tanked permeable sub-base of the pitch.

Indicative SuDS Mitigation Indices for Discharges to Surface Waters			
Type of SuDS Component	Mitigation Indices		
	TSS	Metals	Hydro-carbons
Permeable pavement	0.7	0.6	0.7
Source: Table 26.3 of The SuDS Manual			

Table 3.4 Indicative SuDS Mitigation Indices for Discharges to Surface Waters

3.5.5 Based on the information and assessment set out above, the proposed SuDS features are considered to provide sufficient water treatment for the development proposals.

3.6 Surface water drainage design and management

3.6.1 Proposals are to design the surface water drainage system to accommodate storms up to the 1 in 100 year event plus an allowance of 45% for climate change. The Environment Agency's guidance 'Flood risk assessments: climate change allowances' to support the National Planning Policy Framework, which defines the climate change allowances. Table 3.4 below sets out the peak rainfall allowances for the Medway Management Catchment which the site is located within.

Peak Rainfall Intensity Allowance: Medway Management Catchment			
Rainfall Event	Epoch	Central Allowance	Upper End Allowance
3.3%	2050s	+20%	+35%
	2070s	+20%	+35%
1%	2050s	+20%	+45%
	2070s	+20%	+40%
Source: Environment Agency - 2016			

Table 3.5 Peak Rainfall Intensity Allowance

3.7 Infiltration rates

3.7.1 Infiltration testing has been completed by JPP Geotechnical & Environmental for the proposed AGP, as described in Section 2.6 above, and detailed in **Appendix E**.

3.7.2 Based on the results and recommendations made in the report, a conservative rate of 1.11×10^{-5} m/s (0.03996 m/hr) will be used. This has been taken from the infiltration rates derived from SA01 (see Table 2.2), based on the lower conservative rate observed within the Thanet Formation – Sand underlying the site.

3.8 Infiltration requirements – geocellular soakaway

3.8.1 The proposed drained area of the development is 0.643ha (6,425m²), as identified on the plan enclosed in **Appendix G**. Surface water will be managed via infiltration techniques, utilising a geocellular soakaway located to the north-west corner of the proposed AGP. The soakaway has been designed based on the following parameters.

Rainfall profile	= Flood Estimation Handbook
Return period	= 100 year
Climate change	= 45%
Durations	= 15 to 10080 minutes
Void ratio	= 95%
Build exclusion zone from soakaway	= 10m
Base depth of soakaway	= 3.0m bgl
Depth of soakaway unit	= 1.3m

Depth of cover achieved	= 1.7m
Dimensions	= 24.0 x 8.4 m
Plan area	= 201.6m ²
Volume	= 248.6m ³

- 3.8.2 To benefit from the capacity available within the permeable sub-base of the pitch, surface water flows from the pitch will be restricted prior to connecting into the soakaway. Surface water flows from the pitch will be restricted to 2.0 l/s. This arrangement is represented as a cascade within the design calculations, as enclosed in **Appendix H** for the 1 in 100 year plus 45% climate change scenario.

3.9 Attenuation requirements – permeable sub-base (AGP)

- 3.9.1 Surface water will be attenuated within the permeable sub-base of the pitch. Based on the proposed drained area and designed discharge rate of 2.0 l/s, the storage requirement has been calculated utilising the following parameters.

Rainfall profile	= Flood Estimation Handbook
Drained area	= 6,425m ²
Pitch length	= 97m
Pitch width	= 61m
Fall across pitch (longitudinal)	= 1:300
Depth of permeable sub-base	= 300mm
Porosity of permeable sub-base	= 30 % voids
Control	= Vortex Flow Control

- 3.9.2 Storage calculations for the 1 in 30 year event have been undertaken based on the parameters set out above and below. Full calculations are enclosed in **Appendix I**. The results confirm that the permeable sub-base of the pitch is sufficient to accommodate all the storage required for the 1 in 30 year event.

Return period	= 1 in 30 year
Climate change	= 0%
Durations	= 60 to 10080 minutes
Max volume of flooding triggered	= 0m ³

- 3.9.3 Storage calculations for the 1 in 100 year event have been undertaken based on the parameters set out above and below. Full calculations are enclosed in **Appendix J**. The results confirm that the construction depth of the pitch is sufficient to accommodate all the storage required for the 1 in 100 year event.

Return period	= 1 in 100 year
Climate change	= 0%
Durations	= 60 to 10080 minutes
Max volume of flooding triggered	= 0m ³

3.9.4 Storage calculations for the 1 in 100 year plus 45% climate change event have been undertaken based on the parameters set out above and below. Full calculations are enclosed in **Appendix K**. The results confirm that the construction depth of the pitch is not sufficient to accommodate all the storage required for the 1 in 100 year plus 45% climate change event.

Return period	= 1 in 100 year
Climate change	= 45%
Durations	= 60 to 10080 minutes
Max volume of flooding triggered	= 71.0m ³

3.9.5 The pitch will be surrounded by barriers that will offer containment of the above-ground flooding identified for the 1 in 100 year plus climate change event. This approach is considered acceptable based on the nature of the development.

3.9.6 The indicative surface water drainage layout is shown on the plan enclosed in **Appendix F**.

3.10 Overland flows

3.10.1 Proposals are to design the surface water drainage to accommodate the 1 in 100 year storm event taking into account the predicted future effects of climate. Clearly there is a risk of this storm event being exceeded, albeit this risk is considered very low. In such an event the proposed drainage systems will become overwhelmed and overland flows could occur. Overland flows will be directed to follow the path that overland flows currently follow.

3.10.2 Predicted overland flow routes are shown on the plan enclosed in **Appendix L**.

4.0 Foul water drainage strategy

4.1 There will be no foul water drainage associated with the proposal.

5.0 Maintenance

5.1 Surface water drainage maintenance

5.1.1 The proposed surface water drainage will comprise a private network which will be maintained by Northfleet School for Girls.

5.2 Foul water drainage maintenance

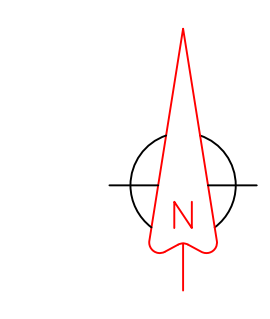
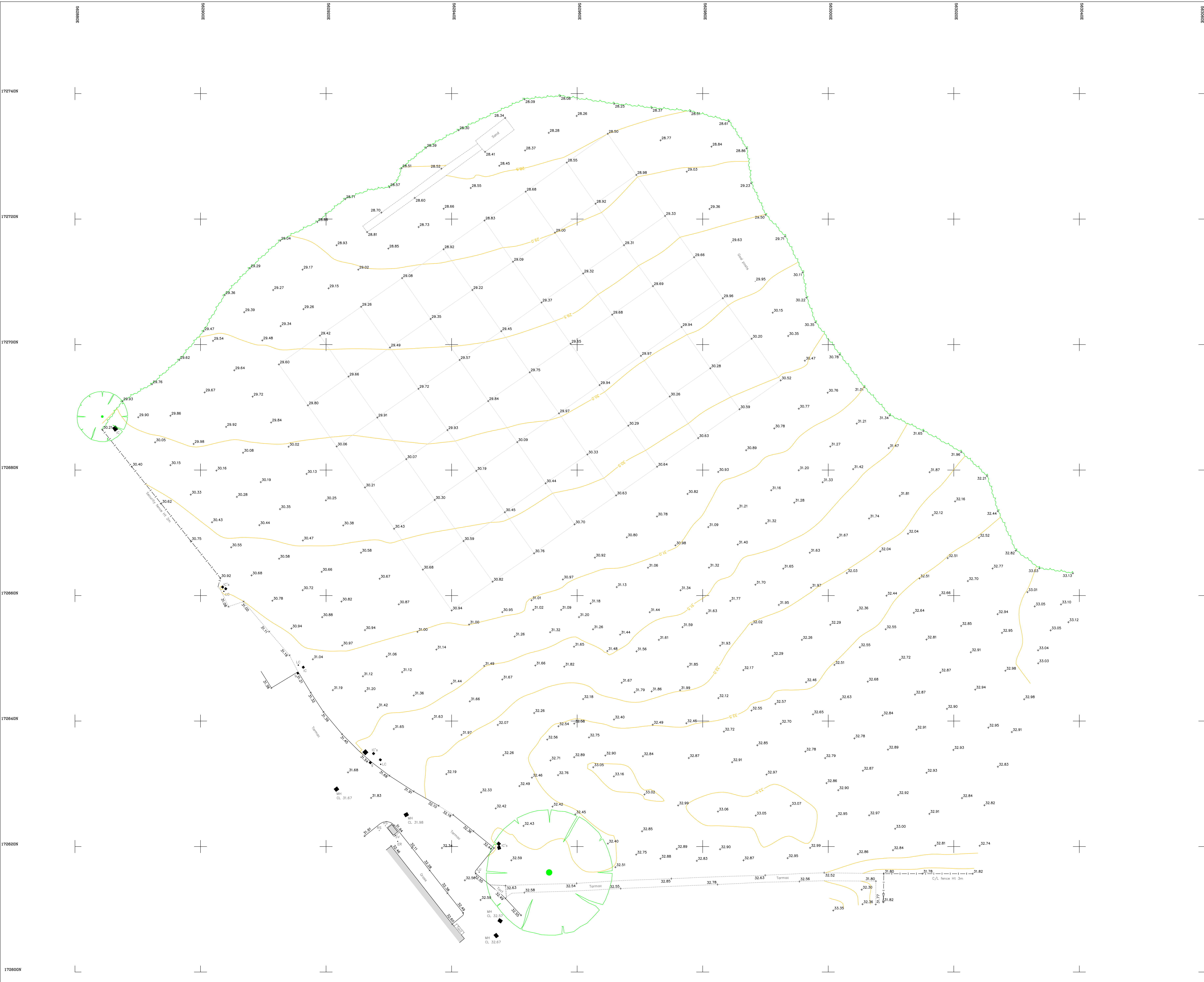
5.2.1 There is no foul drainage associated with the proposal.

6.0 Summary and conclusions

- 6.1 The proposed artificial grass pitch is located at Northfleet School for Girls, Hall Road, Northfleet. The site sits within the eastern part of the wider site for Northfleet School for Girls, with playing fields and courts to the south-east, and school buildings to the west. Hall Road is located to the north of the site, whilst trees are located to the east with Wombwell Hall Care Home beyond.
- 6.2 The proposed development will comprise of an artificial grass pitch.
- 6.3 The Ground Investigation conducted by JPP G&E identified that the site is underlain by Thanet Formation. Based on the infiltration testing undertaken, soakaways are considered to be a viable method for managing surface water at the site. With reference to the chalk dissolution risk set out within the Ground Investigation report, the pitch will be appropriately sealed beneath the sub-base, and the soakaway located in accordance with the 10m offsets required due to the identified chalk dissolution risk.
- 6.4 Surface water from the development will drain via infiltration techniques. Infiltration will be facilitated via a geocellular soakaway, located to the north-west corner of the proposed AGP. To benefit from the capacity available within the permeable sub-base of the pitch, surface water flows from the pitch will be restricted to 2.0 l/s prior to connecting into the soakaway. The permeable sub-base of the pitch will provide interception, treatment and initial attenuation of surface water. The soakaway is designed for the 1 in 100 year plus 45% climate change scenario.
- 6.5 The proposed AGP will include a 0.300m deep permeable sub-base, with a total construction depth of 0.405m. This is shown to be sufficient to accommodate all storms up to and including a 1 in 100 year event. For the 1 in 100 year plus 45% climate change event, 71m³ of above ground flooding is triggered. The pitch will be surrounded by barriers that will offer containment of the above-ground flooding identified for the 1 in 100 year plus climate change event. This approach is considered acceptable based on the nature of the development.
- 6.6 The proposals for a 3G Artificial Turf Pitch at Northfleet School for Girls, Hall Road, Northfleet are therefore fully compliant with policy in respect of development and flood risk, such that flood risk considerations do not constitute a barrier to the granting of planning consent.



Appendix A
Topographical Survey
JPP Surveying drawing no. 27854Y-01



General notes
Grid and levels have been aligned with OS National Grid (OSGB36) (15)
All dimensions and levels are in metres unless noted otherwise.
This plan should only be used for its original purpose. JPP accepts no responsibility if supplied to any other party than the original client.

Key			
AV	Air Valve	FL	Flood Light
Bal	Bollard	FW	Foul Water
BH	Borehole	g	Gully
BL	Bed Level	GV	Gas Valve
BT	BT Cover	HL	Height
CATV	Cable TV Cover	IC	Inspection Chamber
C/B	Close Boarded Fence	IL	Invert Level
C/L	Chain Link Fence	IR	Iron Railing
CL	Cover Level	ko	Kerb Outlet
Col	Column	LC	Lighting column
EI	Elsc	LU	Light unit
ER	Earth Rod	MH	Manhole
EP	Electricity Pole	Mkr	Marker Post
FI	Fire Hydrant	O/H	Overhead
FL	Floor Level	PB	Post Box
		Po	Post
		P/R	Post & Rail Fence
		P/W	Post & Wire Fence
		RE	Rodding Eye
		RS	Road Sign
		rip	Rainwater Pipe
		SV	Stop Valve
		swp	Soil & Vent Pipe
		SW	Surface Water
		TL	Traffic Light
		TP	Telegraph Pole
		TW	Top of wall
		ul	Unable To Lift
		VP	Vent Pipe
		WL	Water Level
		WM	Water Meter
		wo	Wash Out

Building		Bottom of Bank	
Control Station		Top of Bank	
Tree		Vegetation	
Bore Hole		Change of surface	
Gate		Contours	
		OH Telecom	
		OH Electric	

- Infrastructure Design
- Structural Engineering
- Development Planning
- Professional Advice
- Geotechnical & Environmental
- Surveying

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ClientFootball Foundation

ProjectNorthfleet School for Girls

TitleTopographical Survey

Scale at A01:200

Drawn byJJS

Checked byTB

Date8/2/24

Status

Project ref27854Y

Drawing no.01

Revision



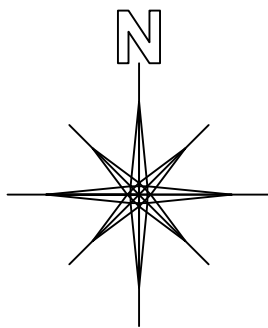
Appendix B
General Arrangement Plan
Labosport Ltd drawing no. 24-004(G-218481) 02

ARTIFICIAL GRASS PITCH (AGP) PROPOSAL



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REFERENCE SHOULD ALSO BE MADE TO HISTORICAL PLANS AND AS BUILT DRAWINGS
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- SYNTHETIC TURF SURFACE
- 1.2M HIGH SPECTATOR FENCING
- 4.5M HIGH PERIMETER FENCING
- DOUBLE GATE
- SINGLE GATE
- STORAGE CONTAINER AREA
- 300MM MOWING STRIP
- 6NO.FLOODLIGHTING COLUMNS TO AGP
- 8NO.FLOODLIGHTING COLUMNS TO EXISTING TENNIS COURTS
- INDICATIVE LANDSCAPING BUND



LEGEND / KEY



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AUTHOR

NORTHFLEET SCHOOL

CLIENT

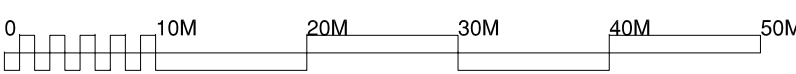
ARTIFICIAL GRASS PITCH (AGP)
DEVELOPMENT

PROJECT

GENERAL ARRANGEMENT

DRAWING

STAGE	LK	01
REVISION	17.05.24	
PROJECT	LSUK-24-004	24-004(G-218481) 02
DATE	1:500	
SCALE		
FIGURE	A1	
FIGURE		





Appendix C
Southern Water Asset Plan



(c) Crown copyright and database rights 2024 Ordnance Survey 100031673
Data updated: 21/11/23

Scale: 1:1250
Map Centre: 562938,172650

Date: 04/01/24
Our Ref: 1358458 - 2

Wastewater Plan A3
Powered by digdat



lydia.crawford@jppuk.net

27677



The positions of pipes shown on this plan are believed to be correct, but Southern Water Services Ltd accept no responsibility in the event of inaccuracy. The actual positions should be determined on site. This plan is produced by Southern Water Services Ltd (c) Crown copyright and database rights 2023 Ordnance Survey 100031673. This map is to be used for the purposes of viewing the location of Southern Water plant only. Any other uses of the map data or further copies is not permitted.

WARNING: BAC pipes are constructed of Bonded Asbestos Cement.

WARNING: Unknown (UNK) materials may include Bonded Asbestos Cement.

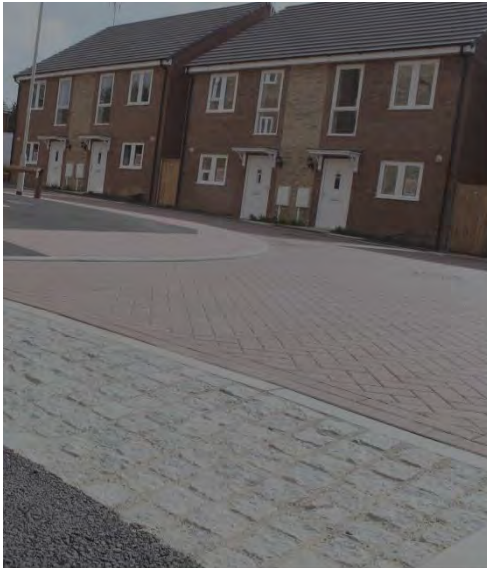


Appendix D
Extract of Ground Investigation
JPP G&E report ref. R-SI-27677-01-00

- Structural Engineering
- Infrastructure Design

- Professional Advice
- Development Planning

- Geotechnical & Environmental
- Surveying



Proposed Artificial Grass Pitch Northfleet School for Girls Hall Road Northfleet DA11 8AQ



Site Investigation Report

Revision 00: March 2024
R-SI-27677-01-00

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1.0 Executive Summary

1.1.1 The following is provided as an overview and should not be relied upon in isolation to the main report.

Summary overview

Existing site	The proposed pitch is located within grassed playing fields located to the east of Northfleet School for Girls school buildings. There is a mature tree adjacent to the south west corner of the proposed pitch and the Root Protection Zone (RPZ) could dictate the positioning of the proposed pitch. There are also trees along the north and east boundaries of the playing fields close to the proposed pitch.
Proposals	Construction of a 3G Artificial Grass Pitch (AGP) with associated fencing, floodlights and drainage infrastructure. There is a significant fall across the proposed pitch footprint from south to north, of between around 3.5 to 4.9m and therefore some levelling earthworks (cut and fill) are anticipated to reduce the fall across the pitch. This includes some presumed artificial raising in the south west corner of the proposed pitch.
History	The location of proposed pitch appears to have remained undeveloped open space associated with Wombwell Hall prior to the playing fields with two circular features with deciduous trees recorded in the south. The school was constructed around 1950 and there appears to be a terraced/lower area in the north of the pitch as possible tennis courts and therefore potential for this to have been filled/relevelled. During the school construction works in 2011, the south west of the pitch appears to have been utilised as a car park and contractors' compound with soils mounded which were subsequently landscaped and now forms a slightly raised area.
UXO	A medium risk was determined by a site-specific risk review, and therefore support by a specialist EOD engineer was required for our intrusive investigations and will be necessary for the construction phase. A detailed UXO desk study report could be undertaken to refine the preliminary risk assessment, which may determine that supervision may not be necessary.
Geology	There are no superficial deposits recorded within or close to the site, with bedrock geology from the surface mapped as Thanet Formation (sand) over Lewes Nodular Chalk Formation, Seaford Chalk Formation and Newhaven Chalk Formation.
Fieldwork	7 windowless sample boreholes to a maximum depth of 2.0m, 6 dynamic probes to a maximum depth of 3.8m and 5 TRL Dynamic cone penetration tests to measure insitu CBR values.
Ground conditions	In general, ground conditions comprised a covering of 0.3 to 0.4m of Topsoil. In WS01, located in the south west corner of the pitch, Made Ground was encountered to 1.0m depth bgl considered to have been artificially raised at the time of the school redevelopment. Beneath the Topsoil, deposits comprised very soft to firm clays in each of the boreholes apart from the south west and south east positions, to depths of between 0.85 to 1.45m bgl which could represent Head deposit or weathered Thanet Formation. Such clay deposits, and beneath the Made Ground in WS01 in the south west and beneath the topsoil in WS02 in the south east, were underlain by a sand or gravel deposit of Thanet Formation, within which each of the boreholes were terminated at 1.0 or 2.0m depth bgl due to the density and SPT refusal (N value of 50).
Groundwater	No groundwater was encountered during the investigation and groundwater is considered likely to be deep, however there is always a potential for shallow perched water to be present.
Chalk Dissolution Hazard and Deneholes	There is considered to be a moderate risk of chalk dissolution and of encountering unrecorded deneholes within the pitch location. Design of the pitch, floodlight foundations and drainage should take into account the risk of chalk dissolution and encountering such features. Should any unusual features such as voiding or particular loose soils be encountered during construction, that could be suggestive of a natural dissolution or manmade denehole feature, then further specialist advice should be sought before proceeding and to determine an appropriate course of action.

Summary overview

Floodlight foundations

Pad foundations are considered suitable with a minimum founding depth of 0.75m (beneath existing or proposed levels, where the deepest should apply following any levelling earthworks) into medium dense sands or gravels of Thanet Formation deposits, through the overlying clay deposits encountered in most positions, and deepening for trees and vegetation based on low volume change potential. Foundations should also be deepened to locate consistent soils at formation level and through any potentially loose deposits.

Larger size pads are recommended to be cautious with respect to the risk of chalk dissolution. Increased levels as part of the levelling earthworks are likely to result in significantly deepened foundations in the north of the pitch which could prove difficult to then excavate to form foundations following the earthworks. Therefore, it is recommended that any impacted floodlight foundations are constructed prior to the earthworks and built up to above the post level earthworks.

Should any unusual features such as voiding or particular loose soils be encountered during construction, that could be suggestive of a natural dissolution or manmade denehole feature, then further specialist advice should be sought before proceeding and to determine an appropriate course of action.

Earthworks

Based on the fall across the pitch of around 3.5 to 4.9m from south to north, some levelling earthworks (cut and fill) are anticipated to reduce the fall. This is also likely to require some sloped banks and retaining features. Retaining features and the levels and gradients of any sloped banks will need to be appropriately designed and considering the risk of slope instability of both the existing and new slope. Floodlighting foundation design and depths should also take into account the position in relation to any formed sloped or retaining features.

Concrete classification

A classification of DS-1 ACEC AC-1 is applicable for the Thanet Formation and Made Ground.

Pitch construction

A CBR value of 2.5% is recommended for the near surface natural soils, based on the clay soils and worst case insitu measurements, for design and to be reassessed during construction and <2% for any Made Ground. However, as the pitch footprint is likely to require some cut and fill earthworks, the CBR will be subject to the compaction earthworks and there is potential for adverse settlement where the pitch may span across variable competency natural deposits which could be more pronounced, and for spanning between natural formation soils and soils placed and compacted, following levelling earthworks. Ground improvement such as lime/cement stabilisation and/or use of geogrid reinforcement may be appropriate to improve the CBR and reduce the potential for adverse settlement to occur where the pitch spans across variable ground conditions both in composition and competency and following levelling earthworks.

Any loose or soft spots identified during the preparation of the formation level should be excavated and replaced with a suitably compacted fill. Any near surface clay soils within the influence of existing trees, containing roots or potentially desiccated should be removed and not reused within the pitch extent to reduce the potential for seasonal shrinkage and swelling.

A root barrier may be appropriate to reduce the potential for root ingress beneath the pitch footprint. The position of the pitch is likely to be dictated by the Root Protection Zones of nearby trees, in particular the tree located close to the south west corner. Specialist advice should be sought in regard to protecting nearby trees from the pitch construction and earthworks.

Infiltration potential/ Surface Water Drainage

Indicative infiltration testing within the granular Thanet Formation did not provide sufficient infiltration to derive a rate. It is possible that the fines (clay/silt) content limited the permeability or smearing of the borehole surface and that deeper deposits or testing in machine pits could show improved infiltration potential and determine the soils to be suitable for soakaways. This would require testing in machine pits to further assess. If soakaways are determined to be a suitable drainage solution, due to the potential risk of chalk dissolution, soakaways should be located sufficient distance (>10m) from any proposed or existing structures (including floodlights), site boundaries, and ideally the pitch footprint, and designed to minimise the risk from chalk dissolution.



Summary overview	
	There are no obvious drainage ditches within and around the site. A surface water sewer is located within Hall Road to the north of the school buildings and the school drainage may outfall into this sewer network. Further enquiries are recommended to establish the existing school drainage and ultimate outfall, whether this be to existing soakaways or to a positive outfall, with a view to connecting into for the proposed pitch or to guide the drainage strategy for the proposed pitch. Any outfall will be subject to seeking necessary approvals and a detailed drainage strategy.
Table 1.1	

3.9.2 As such all-site personnel should be provided with a UXO Safety Awareness Talk before intrusive works commence, and any intrusive work (both site investigation and future construction) should be supported by specialist EOD services to clear working areas as the site is developed.

3.9.3 A detailed UXO risk assessment could be carried out to refine the risk which may reduce the risk and thus requirements for the construction phase.

3.10 Coal mining risk

3.10.1 The site is not located within an area at risk from Coal Mining.

3.11 Chalk dissolution risk, mining cavities and mineral sites

3.11.1 A Mining and Ground Stability report (presented in Appendix D) has been obtained for the site to assess the potential chalk dissolution risk at the site and quarrying/mineral pits on or near to the site.

3.11.2 The report indicates that there is one BGS Recorded Mineral Site within 250m of the site, one within 500m and eleven within 1km. The closest of these is Aspdin Road Denehole, approximately 205m to the south, a now ceased underground operation extracting chalk from the White Chalk Subgroup. The next closest is approximately 270m to the north west, and is at Wombell Road Denehole, also a ceased underground operation extracting chalk from the White Chalk Subgroup.

3.11.3 There is one man made mining cavity within 250m of the site, one within 500m and four within 1km. The commodities extracted were chalk in all of the cavities, either within the Thanet Formation of Chalk Group, with the closest being a chalkwell/Denehole approximately 180m to the south east of the site.

3.11.4 There is one recorded natural cavity located within 1km of the site. It is recorded to be approximately 755m to the south east and relates to six solution pipes.

3.11.5 With reference to the report and associated map defining the ground dissolution hazard (presented in Appendix D), the subject site area is indicated to have no hazard to low risk, along the south west, of chalk dissolution and a moderate risk 175m to the south, becoming high further south. This is likely as a result of the site being mapped as having no superficial deposits with Thanet Formation over the Chalk Formations, whilst from 175m to the south, the Thanet Formation is not mapped over the chalk. This would allow a direct pathway for water to percolate vertically to the chalk and potentially cause dissolution features to occur. However, as the Thanet Formation can be predominantly granular, this could allow a pathway for water to percolate vertically into the chalk and potentially cause dissolution features to occur and along with the potential for unrecorded manmade Denehole features, which are recorded where Thanet Formation is mapped above the chalk, we suggest there to be a **Moderate Risk** of chalk dissolution and manmade Denehole features.

4.0 Fieldwork

- 4.1.1 Fieldwork was carried out on the 9th February 2024 and comprised the following:
- 7 Windowless sample boreholes to 1.00 and 2.00m depth bgl due to the density of the ground conditions
 - 6 Dynamic probes to a maximum of 3.80m bgl, limited by the UXO clearance
 - 5 TRL Dynamic Cone Penetration tests to measure insitu CBR values.
- 4.1.2 Due to the medium risk of encountering buried UXO within the site, an EOD specialist engineer was present during our works, clearing each position prior and during the drilling process. The final drilled depth of the dynamic probes was limited by the clearance depth of the magnetometer, which could safely clear in a maximum of 2.0m increments. Where the magnetometer gives a slightly unclear reading, clearance can only be given in 1.0m increments. The dynamic probes were therefore only cleared up to 2.0m below the final depth of the adjacent boreholes, limiting the final depth of the probes to between 2.0m and 3.8m bgl.
- 4.1.3 Each position was scanned using a cable avoidance tool and signal generator prior to positioning and proceeding. Utilities searches were also obtained prior to the fieldwork to confirm the position of potential underground services (excluding private services). These are presented in Appendix F.
- 4.1.4 The approximate location of each exploratory hole position is shown on the plan (Drawing 01) and included within Appendix A.
- 4.1.5 The borehole positions were targeted outside of existing marked pitches.

5.0 Ground Conditions

5.1 Soils

Topsoil

- 5.1.1 Each of the exploratory boreholes encountered a brown slightly gravelly, slightly silty, sandy topsoil to between 0.30m and 0.40m bgl. Gravel generally comprised of sandstone, with brick encountered in WS04 and wood encountered in WS01.

Made Ground

- 5.1.2 Underlying the topsoil within WS01 (south west corner where the ground appears to have been raised at the time of the school construction works in 2011), Made Ground was encountered to 1.00m bgl. These soils initially comprised of a light greyish brown, sandy sandstone gravel to 0.40m bgl, over a brown, slightly gravelly, slightly sandy, silty clay with some roots and rootlets. The gravels within the clay comprised of sandstone, concrete and brick.

Possible Head Deposits/ Weathered Thanet Formation

- 5.1.3 Underlying the topsoil, in each of the positions apart from WS01 in the south west corner and WS02 in the south east corner, encountered presumed natural deposits comprising a very soft to firm slightly gravelly, sandy to very sandy, silty CLAY to depths of between 0.85m to 1.45m bgl. Gravels consisted of sandstone and flint, although the gravel content decreased towards the north of the proposed pitch. Such deposits have been considered to potentially be weathered Thanet Formation however these could represent a Head or other superficial deposit based on the low strength and flint gravels.

Thanet Formation

- 5.1.4 In the south of the proposed pitch, within WS01 and WS02, no clay was encountered underlying the topsoil or Made Ground in WS01. Underlying the topsoil within WS02 was a light yellowish brown, silty, sandy, sandstone and flint GRAVEL to 0.95m bgl, becoming clayey to the base of the borehole at 1.00m bgl where the borehole was terminated due to the density. The silty gravel was also encountered underlying the Made Ground in WS01 and beneath the clay from 0.85m depth within WS04 (located in the north east corner) to the base of the exploratory boreholes.
- 5.1.5 Underlying the clay within WS03 and WS05-07 to the base of the exploratory boreholes, an orangish brown and occasionally grey clayey, fine to medium SAND was encountered, with occasional clay lenses noted within the sand in WS03.
- 5.1.6 Hand pocket penetrometer tests converted to kPa recorded strength values of between 10 and 70kPa within the cohesive soils, indicating very low to medium strength deposits.

5.1.7 Standard Penetration Test (SPT) 'N' values undertaken recorded lowest values of 5 and 8 at 0.50m depth within the clay soils in WS06 and WS07. At 1.0m depth in WS03, located centrally along the east side of the proposed pitch, an SPT N value was measured as 26 with the clay deposits, which was the deepest borehole achieved. All other positions refused with SPT N values of 50 for 300mm penetration or 50 for less than the full test penetration of 300mm at 1.0m depth and in WS03 at 2.0m depth.

Dynamic probes

5.1.8 Dynamic Probe DP1, undertaken adjacent to WS01 showed relatively consistent blow counts of 3 blows per 100mm to 1.20m, with exceptions at 0.60m, 1.00m and 1.10m depth where 11, 32 and 2 blows per 100mm respectively were recorded. The low blow counts with localised increases are consistent with the encountered Made Ground with gravels, with the blow counts then increasing to 25 blows per 100mm at 1.60m bgl, where the probe rods kicked off from centre, resulting in an early termination of the test. The increase in blow counts correlates with the very dense gravels of the Thanet Formation.

5.1.9 Dynamic Probe DP2, undertaken adjacent to WS02 was completed to 2.00m bgl due to the EOD engineer being unable to clear to any greater depths. This probe showed low blow counts of 2 blows per 100mm to 0.40m bgl, the approximate base of the topsoil, increasing to generally between 5 and 12 blows per 100mm, with a peak of 15 at 1.10m bgl.

5.1.10 Dynamic Probe DP3 was undertaken adjacent to WS03 in the east of the proposed pitch, and was completed to 3.80m bgl before kicking off on a presumed cobble, although the EOD engineer could only clear to a maximum of 4.00m bgl. Once again, initial very low blow counts of between 0 and 1 blows per 100mm were recorded through the topsoil and into the initially soft underlying clays, increasing to between 2 and 3 blows per 100mm to 1.50m bgl, where the sands of the Thanet Formation were encountered. Blow counts then consistently increased to 18 blows per 100mm at 3.80m bgl, with a separate peak at 2.00m bgl of 14 blows per 100mm.

5.1.11 Dynamic Probe DP4, adjacent to WS04 recorded blow counts of 0 and 1 blows per 100mm to 0.90m bgl through the encountered soft clays, increasing to between 7 and 10 blows per 100mm by 2.00m bgl, where the test was terminated due to lack of UXO clearance.

5.1.12 Dynamic Probes DP5 and DP6, adjacent to WS05 and WS07 respectively were broadly similar, showing blow counts of between 0 and 2 blows per 100mm to 1.10m and 1.30m bgl respectively, increasing to 15 and 13 blows per 100mm by 1.40m and 2.00m bgl. Blow counts then decreased to as low as 3 blows per 100mm in DP05 and 4 blows per 100mm in DP6 by 2.40m and 2.60m bgl, increasing to 8 blows per 100mm by 3.00m bgl, the deepest the EOD engineer could clear the probes to.

5.2 Groundwater

- 5.2.1 No groundwater was encountered during the investigation and is considered likely to be deep however there is always a potential for shallow perched groundwater to be present.

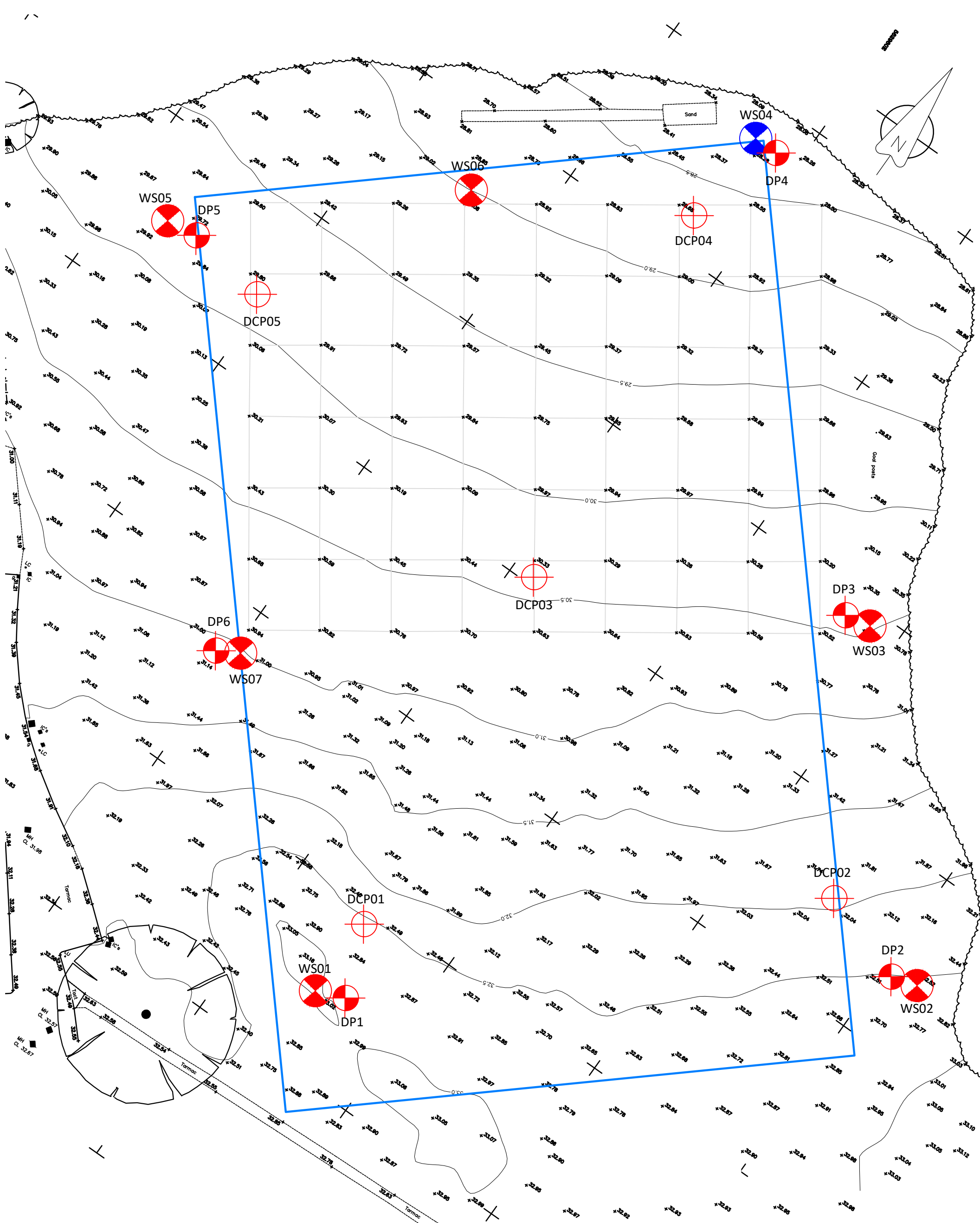
7.7 Earthworks

- 7.7.1 The topographical survey shows there to be a fairly significant fall across the proposed pitch footprint to the north, from a high of approximately 33.16m AOD in the south west corner (within the presumed artificially raised area) and approximately 32.85m in the south east corner to approximately 28.25m AOD in the north east corner. Suggesting a fall of approximately 4.9m from south east to north east (diagonally across the proposed pitch footprint) and 4.6m from the south east to the north east along the eastern side of the proposed pitch. The south west corner of the pitch appears to have been artificially raised by around 1.0m height which is considered likely to be formed from arisings during the school redevelopment in 2011.
- 7.7.2 Based on the fall across the pitch of around 3.5 to 4.9m from south to north, some levelling earthworks (cut and fill) are anticipated to reduce the fall. This is also likely to require some sloped banks and retaining features. Retaining features and the levels and gradients of any sloped banks will need to be appropriately designed and considering the risk of slope instability of both the existing and new slope. Floodlighting foundation design and depths should also take into account the position in relation to any formed sloped or retaining features. The slopes created will also need to consider the presence of trees and associated Root Protection Zones. Specialist advice should be sought in regard to protecting nearby trees from the pitch construction and earthworks.
- 7.7.3 The upper clay soils encountered on site are likely to soften extremely quickly when exposed to periods of sustained wet weather and where over trafficked. Care should be taken to work on the exposed soils in periods of dry weather and to not excessively track over the exposed subsoil/formation layer prior to laying the subbase, otherwise softening/dilation could occur which would lower the CBR values of the underlying soils. This should also be accounted for during the proposed earthworks and any excavated and exposed soils will need to be sufficiently protected.
- 7.7.4 Use of any excavated material as fill material will be subject to classification, grading and appropriate compaction following Series 600: Earthworks. Any topsoil/organic soil is not suitable for reuse as fill.

7.8 Infiltration potential and surface water drainage

- 7.8.1 Infiltration testing was carried out in WS04, the lowest point of the grassed playing fields in the north east corner, with an 180mm fall in water levels over 143 minutes, remaining more than 50% full and therefore insufficient to derive an infiltration rate however suggests that there could be some infiltration potential. It is thought that the fines (clay/silt) content may have limited the infiltration and the effect of smearing of these fines on the sides of the borehole may have also reduced the infiltration. The granular soils encountered may therefore provide some infiltration potential however this would be subject to further testing in machine excavated trial pits to facilitate BRE 365 compliant soakaway testing.

- 7.8.2 Borehole soakaways could also be an option with borehole soakaway testing required at specific locations to assess this and derive rates for design. However, these produce concentrated ingress which therefore produce a risk of forming dissolution features. Discharge into chalk, which is a principal aquifer and also a Source Protection Zone 2, is likely to be subject to approval of any design and pollution prevention controls by the Environment Agency. Chalk was not encountered during our investigations and is considered likely to be at significant depth.
- 7.8.3 Due to the risk of chalk dissolution, particularly where granular deposits overlie chalk and therefore a potential pathway for water to enter the chalk, it is recommended that any soakaway drainage is located remote from floodlight foundations and any other structures. Due to the variation in fines and granular content of deposits encountered by our investigations, soakaways as trenches beneath the pitch could result in localised/concentrated infiltration and therefore it is recommended that any such drainage is also located remote from the pitch or constructed with a geogrid reinforcement or crate system to reduce the potential for loss of support should any such features form or be present.
- 7.8.4 Concentrated ingress of water into chalk can create new dissolution features so it is important that drainage is sufficiently designed for this not to occur. It is suggested that drains should be sealed flexible joints or with concrete bedding and haunching to minimise the risk of future damage. Following CIRIA C574, the guidance suggests that where chalk is of low density (assumed to be the case for the site in the absence of testing to confirm otherwise) any soakaways/infiltration system into the ground should be sited at least 10m from foundations, structures and boundaries.
- 7.8.5 With reference to Southern Water records (included within Appendix F), a surface water drain is located off Hall Road to the north of the site, travelling along the eastern side of residential properties which could provide a potential outfall for surface water drainage subject to seeking approval of any drainage strategy.
- 7.8.6 No obvious drainage ditches were observed on or adjacent to site. Southern Water records show a surface water gravity sewer within Hall Road to the north east of the proposed pitch, north of the school buildings and a surface water gravity sewer also within Hall Road to the north east of the pitch. The school drainage may outfall into this sewer network. Further enquiries are recommended to establish the existing school drainage and ultimate outfall, whether this be to existing soakaways or to a positive outfall, with a view to connecting into for the proposed pitch or to guide the drainage strategy for the proposed pitch. Any outfall will be subject to seeking necessary approvals and a detailed drainage strategy.



KEY



WS



WS



DCP



DP

Windowless sample borehole

Windowless sample borehole with infiltration test

TRL - Dynamic cone penetration test

Dynamic probe



Approximate position of proposed 3G AGP pitch full extent 103 x 64m

Base Drawing:
Topographical survey by JPP Surveying.
dated: 09/02/2024, drawing number: 27854Y.



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- Infrastructure Design
- Structural Engineering
- Development Planning
- Professional Advice
- Geotechnical & Environmental
- Surveying

Client The Football Foundation

Project Northfleet School for Girls, Northfleet

Title Investigation Positions over Topographical Survey

Scale at A3 NTS	Drawn by LB	Checked by AP	Date March 2024
Status	Project ref 27677	Drawing no. 01	Revision

JPP QA Document T07 R1

Key to Exploratory Hole Logs

General

Descriptions for soils and rocks generally following:

BS 5930:2015+A1:2020

BS EN ISO 14688-2:2004

BS EN ISO 14689-1:2003

Sampling

ES	Environmental Sample (taken in appropriate sampling container)
D	Disturbed Sample
B	Bulk Sample
C	Core Sample
U	Undisturbed Sample (number of blows indicated in results column)
UT	Thinwalled Undisturbed (number of blows indicated in results column)
W	Water Sample

Insitu Tests

SPT	Standard Penetration Test
SPT (C)	Cone Penetration Test
PID	Photo Ionisation Detector Results (ppm)
PP	Pocket Penetrometer reading converted to shear strength kPa
HV	Converted Shear Vane measurement kPa

Drilling Records

▼	Depth to standing water level
▽	Depth to water strike
TCR	Total Core Recovery (%)
SCR	Solid Core Recovery (%)
RQD	Rock Quality Designation (%)

Backfill Symbols

Arisings	
Concrete	
Blacktop	
Bentonite Seal	
Gravel Filter	
Sand Filter	

Pipe Symbols

Plain Pipe	
Slotted Pipe	
Filter Tip	

Principal Soil Types

Topsoil	
Made Ground	
Clay	
Silt	
Sand	
Gravel	
Peat	

Principal Rock Types

Mudstone	
Siltstone	
Sandstone	
Limestone	
Chalk	

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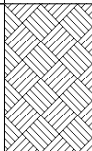
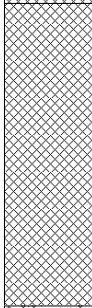
Key to Exploratory Hole Logs





Window Sample Log

Project Name: Northfleet School for Girls		Client: Football Foundation		Date: 09/02/2024	
Location: Northfleet		Contractor: Keystone Site Solutions Ltd			
Project No. : 27677		Crew Name:		Drilling Equipment: Premier 110	
Borehole Number WS01	Hole Type WLS	Level	Logged By TM	Scale 1:15	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
		0.60	ES	N=50 (8,8/10,12,14,14)	0.30			Brown slightly gravelly slightly silty sandy TOPSOIL with some roots and rootlets. Gravel consists of subangular to angular fine sandstone and wood.	1
							0.40		
		1.00						Brown slightly gravelly slightly sandy silty CLAY with some roots and rootlets. Gravel consists of subangular to angular fine to medium sandstone, concrete and brick. (MADE GROUND)	
							1.20		
		End of Borehole at 1.200m							

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
Remarks No groundwater encountered.											



Window Sample Log

Project Name: Northfleet School for Girls		Client: Football Foundation		Date: 09/02/2024	
Location: Northfleet		Contractor: Keystone Site Solutions Ltd			
Project No. : 27677		Crew Name:		Drilling Equipment: Premier 110	
Borehole Number WS02	Hole Type WLS	Level	Logged By TM	Scale 1:15	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
			D		0.35			Brown slightly gravelly slightly silty sandy TOPSOIL with some roots and rootlets. Gravel consists of subangular to angular fine sandstone.	1
		0.70						Light yellowish brown silty sandy subangular to angular fine to coarse sandstone and flint GRAVEL. (THANET FORMATION)	
		1.00		N=50 (8,10/12,12,13,13)	0.95 1.00			Very dense light yellowish brown clayey sandy subangular to angular fine to coarse sandstone and flint GRAVEL. (THANET FORMATION) End of Borehole at 1.000m	
									2
									3

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation

Remarks	
No groundwater encountered.	



Window Sample Log

Project Name: Northfleet School for Girls		Client: Football Foundation		Date: 09/02/2024	
Location: Northfleet		Contractor: Keystone Site Solutions Ltd			
Project No. : 27677		Crew Name:		Drilling Equipment: Premier 110	
Borehole Number WS03	Hole Type WLS	Level	Logged By TM	Scale 1:15	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
		0.10	ES		0.40			Brown slightly gravelly slightly silty sandy TOPSOIL with some roots and rootlets. Gravel consists of subangular to angular fine sandstone.	1
		0.50	D					Soft becoming firm orangish brown slightly gravelly sandy silty CLAY with roots and rootlets. Gravel consists of subrounded to angular fine to medium sandstone and flint. (THANET FORMATION)	
		0.70	PP	20.00					
		0.90	PP	70.00					
		1.00		N=26 (4,4/5,6,7,8)					
		1.50	D		1.45			Very dense orangish brown and occasional grey silty fine to medium SAND with occasional clayey lenses. (THANET FORMATION)	2
		2.00		N=50 (8,10/12,12,13,13)	2.00			End of Borehole at 2.000m	
									3

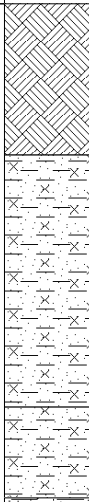
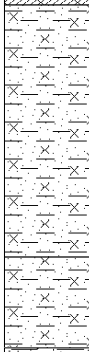
Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation

Remarks	
No groundwater encountered.	



Remarks	
No groundwater encountered.	

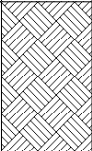
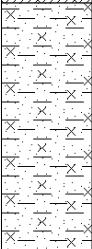
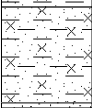


Project Name: Northfleet School for Girls				Client: Football Foundation				Date: 09/02/2024							
Location: Northfleet				Contractor: Keystone Site Solutions Ltd											
Project No. : 27677				Crew Name:				Drilling Equipment: Premier 110							
Borehole Number WS05		Hole Type WLS		Level		Logged By TM		Scale 1:15		Page Number Sheet 1 of 1					
Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description							
		Depth (m)	Type	Results											
		0.50	D	15.00	0.30			Brown slightly gravelly slightly silty sandy TOPSOIL with some roots and rootlets. Gravel consists of subangular to angular fine sandstone.		1 –					
								Very soft orangish brown very sandy silty CLAY. (THANET FORMATION)							
								0.70	PP		55.00	0.80		Firm orangish brown sandy silty CLAY. (THANET FORMATION)	
														Very dense orangish brown slightly clayey fine to medium SAND. (THANET FORMATION)	
														End of Borehole at 1.000m	
		1.00	N=50 (12,13/50 for 270mm)	0.98 1.00					2 –						
										3 –					
Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation							
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation				
Remarks															
No groundwater encountered.															



Window Sample Log

Project Name: Northfleet School for Girls		Client: Football Foundation		Date: 09/02/2024	
Location: Northfleet		Contractor: Keystone Site Solutions Ltd			
Project No. : 27677		Crew Name:		Drilling Equipment: Premier 110	
Borehole Number WS06	Hole Type WLS	Level	Logged By TM	Scale 1:15	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
								Brown slightly gravelly slightly silty sandy TOPSOIL with some roots and rootlets. Gravel consists of subangular to angular fine sandstone.	1
					0.30				
		0.50		N=8 (0,0/2,2,2,2)				Very soft orangish brown very sandy silty CLAY. (THANET FORMATION)	
		0.70	PP	20.00		0.80		Firm orangish brown sandy silty CLAY. (THANET FORMATION)	
		0.90 0.90 1.00	D PP	50.00 N=50 (9,9/50 for 265mm)	0.99 1.00			Very dense orangish brown slightly clayey fine to medium SAND. (THANET FORMATION) End of Borehole at 1.000m	
									2
									3

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
Remarks No groundwater encountered.											



Remarks
No groundwater encountered.

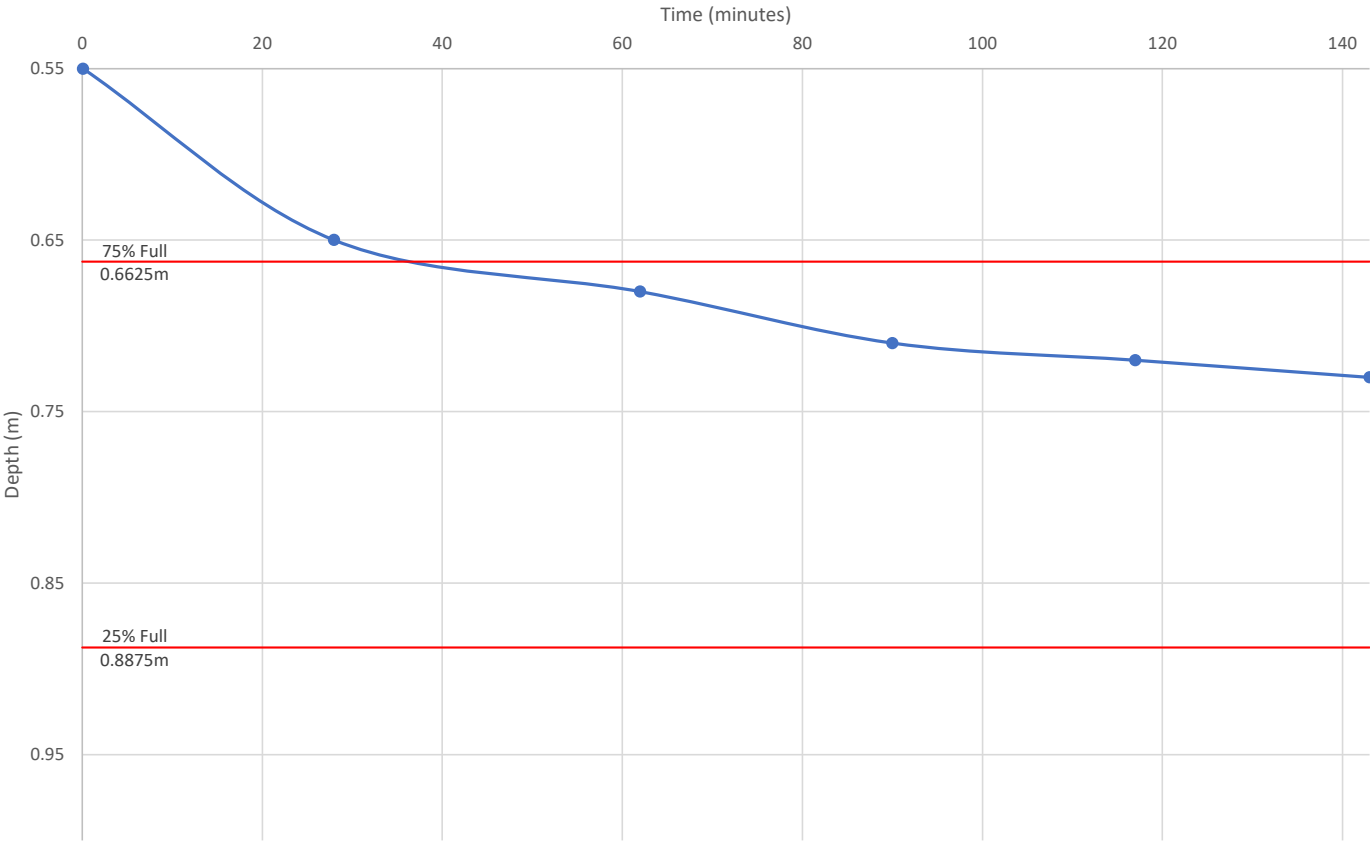
Indicative Soakage Test Results



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Project Name:	Northfleet School for Girls
Project No:	27677
Client:	Football Foundation
Date:	09 February 2024
Revision:	0

WS04 - Cycle 1



Borehole radius 0.05m
Borehole diameter 0.1m
Borehole depth 1m

Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

Insufficient soakage achieved to derive rate

V_{p75-25} : the effective storage volume of water in the soakage borehole between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage borehole up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth



Appendix E
Infiltration Testing Report
JPP G&E report ref. 27677/LR/LC/L01

- Infrastructure Design
- Development Planning
- Geotechnical & Environmental
- Structural Engineering
- Professional Advice
- Surveying



Our Ref: 27677/LR/LC/L01

Your Ref:

Date: 9th June 2025

JPP Geotechnical & Environmental Ltd
Grand Union Works | Whilton Locks
Daventry | NN11 2NH

T: 01604 781811
E: mail@jppuk.net
W: jppuk.net

Labosport UK
Unit 3 Aerial Way
Hucknall
Nottingham
NG15 6DW
FAO Bob Froud

Dear Bob,

Re: BRE 365 Infiltration Testing for a Proposed 3G AGP at Northfleet School for Girls

1.0 Introduction and Scope

- 1.1 JPP were appointed by Labosport Ltd to carry out infiltration testing in machine excavated trial pits following *BRE 365 Soakaway Design* guidance to further inform the surface water drainage strategy for the proposed 3G Artificial Grass Pitch (AGP).
- 1.2 We originally carried out an investigation for the proposed 3G AGP, in February 2024 for the Football Foundation in conjunction with the Football Foundation AGP Framework. This report, ref R-SI-27677-01-00 dated March 2024, should be referred to for additional detail.
- 1.3 This letter report has been produced for the benefit of Labosport Ltd to be utilised in conjunction with the Football Foundation AGP Framework, JPP do not accept liability for the third-party use of the information herein without prior reliance agreed. This report is valid for 6 years from the date of issue.
- 1.4 Investigations have been carried out using reasonable care and judgement based on the scope. There is a potential for ground conditions to vary from those encountered during our investigations and differ where not exposed by our investigations.

JPP Geotechnical and Environmental Ltd Registered in England 11117245

Registered office

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Branksome Business Park
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Dorset | BH12 1ED
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2.0 Fieldwork

2.1 Our fieldwork was carried out on the 28th April 2025 and comprised the following:

- 4 machine excavated trial pits (SA01 to SA04) to depths of between 1.5m and 3.0m bgl
- Infiltration testing within each of the trial pits, in general accordance with *BRE 365 'Soakaway Design'* guidance.

An EOD engineer supervised the excavations to clear for potential buried UXO.

2.2 Each position was scanned using a cable avoidance tool with the use of a signal generator prior to positioning and proceeding. Statutory undertaker's service records were obtained prior to works commencing and were referred to for positioning of the excavations. These exclude private services.

2.3 The positions were targeted in the north west of the playing fields, to be located at the most likely position for the proposed soakaway, being down gradient/slope of the proposed pitch and sufficient offset due to potential chalk dissolution risk.

2.4 Trial pit logs, photographic records and infiltration test records are appended to this letter, together with a plan showing the approximate positions of the tests.

3.0 Site Description and Geology

3.1 The proposed pitch is located within north east of grassed playing field to the east of Northfleet School for Girls. There is a long jump directly to the north of the proposed pitch and a narrow wooded area to the north of this, between the playing field and Hall Road. The wooded area also extends around to the north east and east. The machine excavations were targeted towards the north west edge of the playing field, close to the trees. There is a grassed strip/extent of the playing fields between the proposed pitch location and the car park and access road to the west.

3.2 BGS mapping of the site indicates there to be no superficial within or around the site with bedrock geology from the surface of the Thanet Formation (sands), which is overlying the Lewes Nodular Chalk Formation, Seaford Chalk Formation and Newhaven Chalk Formation (Chalk).

3.3 Our previous windowless sampler boreholes encountered a covering of 0.3 to 0.4m of Topsoil. In WS01, located in the south west corner of the proposed pitch, Made Ground was encountered to 1.0m depth bgl considered to have been artificially raised at the time of the school redevelopment. Beneath the Topsoil, deposits comprised very soft to firm clays in each of the boreholes apart from the south west and south east positions, to depths of between 0.85 to 1.45m bgl which could represent Head deposit or weathered Thanet Formation. Such clay deposits, and beneath the Made Ground in WS01 in the south west and beneath the topsoil in WS02 in the south east, were underlain by a sand or gravel deposit of Thanet Formation, within which each of the boreholes were terminated at 1.0 or 2.0m depth bgl due to the density and SPT refusal (N value of 50).

4.0 Ground Conditions

- 4.1 Each trial pit encountered a covering of Topsoil to depths of between 0.15m to 0.30m bgl, generally comprising of a brown (pinkish brown In SA01) slightly gravelly silty sandy clayey Topsoil with frequent fine to medium flint gravel, and occasional sandstone, glass and brick fragments.
- 4.2 Beneath the Topsoil, were deposits considered to be representative of the Thanet Formation, initially comprising of an orange brown slightly gravelly slightly clayey slightly silty fine to medium Sand, with fine to medium rounded to subrounded to rounded flint to depths of between 0.95 to 1.45m bgl. Beneath this, in each of the four excavations was a layer between 0.2 to 0.35m thick of orange brown slightly silty slightly clayey very sandy Gravel of flint to depths of between 1.2 to 1.7m. This was above a generally orange brown slightly clayey to clayey silty slightly gravelly to very gravelly fine to medium Sand with occasional clayey pockets, to the base depth of the excavations which were terminated at 1.5m and 2.0m bgl, in SA03 and SA02 respectively. In the deeper positions, SA01 and SA04, such deposits were encountered to depths of 2.1 and 2.2m bgl onto a light yellowish brown slightly clayey slightly gravelly silty fine Sand with gravels of flint, encountered to the base depth of the excavations of 2.45m in SA04
- 4.3 Instability of the trial pit sides was experienced in each of the four positions, with occasional collapse of the side walls noted during the soakaway testing and between the testing. This resulted in the shallowing of the positions during the testing.
- 4.4 No groundwater was encountered within the intrusive investigation works undertaken in both February 2024 and April 2025 to depths of up to 3.0m bgl. Groundwater was also not encountered within the exploratory excavations carried out in April and May 2019 for the proposed Sixth Form Block in the south west of the site, with the deepest borehole to 10.0m depth bgl. It is considered likely that groundwater will be reasonably deep.

5.0 Infiltration Testing Results

- 5.1 Infiltration testing was carried out in general accordance with BRE 365 'Soakaway Design', with the observations summarised in the following table. As the rates were slow, it was not possible to complete three full cycles and one complete cycle in 3 of the 4 positions.

Trial pit reference	Cycle	Base depth of trial pit	Test result /observations
SA01	1	2.77m bgl	2.39×10^{-5} m/s
	2	2.85m bgl	1.24×10^{-5} m/s
	3	2.60m bgl	1.11×10^{-5} m/s Conservatively extrapolated after 40 minutes to derive an indicative rate.
SA02	1	2.00m bgl	Insufficient infiltration to derive rate
SA03	1	1.60m bgl	4.50×10^{-6} m/s Conservatively extrapolated after 40 minutes to derive an indicative rate.
SA04	1	2.45m bgl	5.28×10^{-6} m/s Conservatively extrapolated after 40 minutes to derive an indicative rate.

Table 5.1

- 5.2 Instability of the side walls was experienced and some outbreak and infilling of the base of the excavations, resulting in a slight shallowing of the base depth during the testing. Within trial pit SA01, some of the infilled soils were excavated from the base before carrying out the second cycle.

6.0 Assessment and Conclusion

- 6.1 Based on the results of the infiltration testing, the near surface Thanet Formation deposits provide some infiltration potential which improves with depth. Testing within the deepest trial pit, excavated to 3.0m depth bgl, with the base depth of the testing of between 2.85 to 2.6m due to instability and outbreak of the side walls infilling the excavation during and between the tests, produced infiltration rates of between 2.39×10^{-5} to 1.11×10^{-5} m/s. With the third cycles conservatively extrapolated to derive a rate. It is therefore recommended that the proposed soakaway has a base depth of at least 3.0m bgl, into the light yellowish brown slightly clayey slightly gravelly silty fine Sand, and the conservative lowest rate is adopted for design of 1.11×10^{-5} m/s. The trial pit logs and infiltration test results, should be referred to for full details of the ground conditions encountered at the depths of the testing to target for the soakaway design.
- 6.2 This suggested design rate is further supported by the soakaway testing and design rate utilised for the recently constructed soakaway for the Sixth Form block located to the south west, where a geocellular soakaway was installed in the south of the playing fields in 2021, designed using an infiltration rate of 1.38×10^{-5} m/s, based on results of soakaway testing carried out. Ground conditions were similar to those encountered in the proposed soakaway location, based on the fieldwork records. This is also located at a similar topographical level to the proposed soakaway location for the pitch, with the site being on a local high point and levels falling to the north boundary across the pitch and falling to the south, beyond the proposed pitch to the south.
- 6.3 No groundwater was encountered within the intrusive investigation works undertaken in both February 2024 and April 2025 to depths of up to 3.0m bgl. Groundwater was also not encountered within the exploratory excavations carried out in April and May 2019 for the proposed Sixth Form Block in the south west of the site, with the deepest borehole to 10.0m depth bgl. It is considered likely that groundwater will be reasonably deep.
- 6.4 As noted in our previous site investigation report, ref R-SI-27677-01-00, which should be referred to for detailed information, we have considered there to be a potential risk of chalk dissolution. Since the previous investigations, we have now located some existing borehole information from the construction of the Sixth Form building to the south west. The deepest boreholes encountered Thanet Formation deposits to at least 10.0m depth bgl. Although this significant thickness of overlying Thanet Formation does reduce the potential for chalk dissolution, such deposits are permeable and therefore it is not possible to fully discount the potential for vertical permeation of water through the Thanet Formation resulting in chalk dissolution at depth, which could result in infilling of overlying soils and loss of support near surface. There are not however any nearby occurrences of chalk dissolution, with the closest some 755m to the south east recorded as six solution pipes. This significant covering of Thanet Formation above the chalk does however reduce the likelihood of unrecorded manmade features such as Deneholes or chalk wells within the site.
- 6.5 We would still suggest that a cautious approach is maintained of locating the soakaway at least 10m from foundations, structures (including the pitch) and boundaries, based on CIRIA 574 'Engineering in Chalk', where the chalk is of low density (assumed to be the case for the site in the absence of encountering the Chalk deposits within the excavations to allow testing to confirm otherwise).

- 6.6 For the soakaway design, as large and level area as possible is recommended to increase the surface area and targeting the most permeable deposits along its length, with ground conditions varying in fines and granular content and to minimise the potential for concentrated ingress occurring. A crate/geocellular type system as opposed to gravel is recommended to minimise potential for loss into a dissolution feature. It is recommended that drains should include sealed flexible joints or with concrete bedding and haunching to minimise the risk of future damage.
- 6.7 Discharge into the Thanet Formation overlying the Chalk, which is a principal aquifer and within a Source Protection Zone 2 (SPZ2), may be subject to approval of any design and pollution prevention controls, by the Environment Agency.

Yours sincerely,



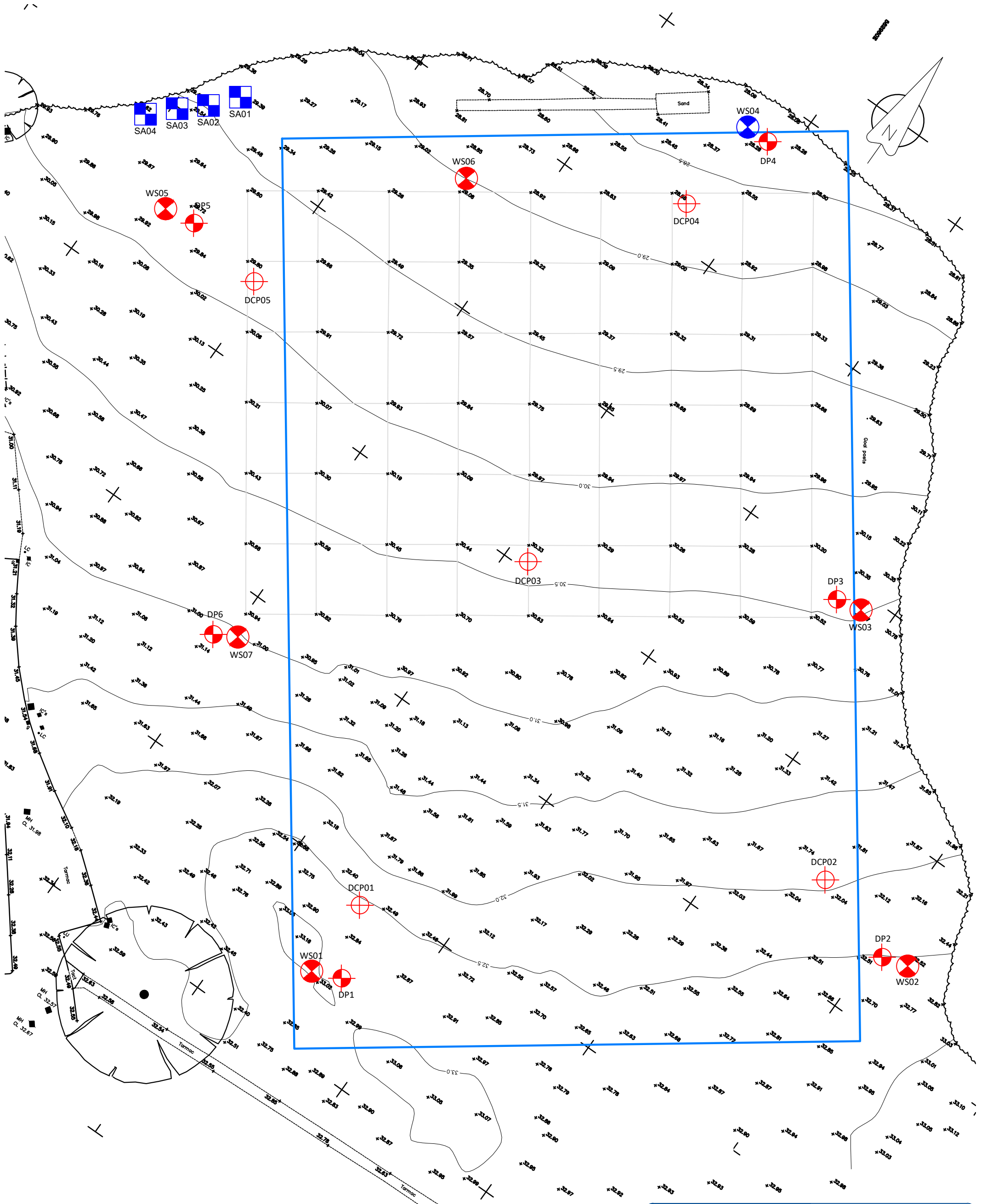
Libby Rogers BSc (Hons) FGS
Geo-Environmental Engineer



Lydia Crawford BSc (Hons) CEnv MEnvSc FGS
Director

Attachments:

Plan to show approximate infiltration test positions
Trial pit logs
Photographic records of trial pits
Infiltration test records



KEY

- WS Windowless sample borehole
- WS Windowless sample borehole with infiltration test
- DCP TRL - Dynamic cone penetration test
- DP Dynamic probe

- SA Machine excavated trial pit with infiltration testing
- Approximate position of proposed 3G AGP pitch full extent 103 x 64m

Base Drawing:
Topographical survey by JPP Surveying.
dated: 09/02/2024, drawing number: 27854Y.
Revision A: To include soakaway testing positions
carried out in April 2025.

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Client **Labosport**

Project **Northfleet School for Girls, Northfleet**

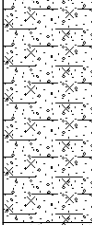
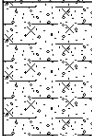
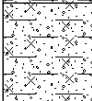
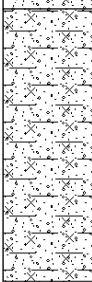
Title **Investigation Positions over Topographical Survey**

Scale at A3	NTS	Drawn by	LB	Checked by	AP	Date	April 2025
Status		Project ref	27677	Drawing no.	01	Revision	A

JPP QA Document T07 R1



Trial Pit Log

Project Name: Northfleet School for Girls		Client: Labopsort Ltd.			Date: 28/04/2025						
Location:		Contractor:									
Project No. : 27677		Crew Name:			Equipment: 3T Excavator.						
Location Number SA01		Location Type TP		Level		Logged By LR		Scale 1:25		Page Number Sheet 1 of 1	
Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description			
		Depth (m)	Type	Results							
					0.15			Grass over pinkish brown slightly clayey slightly gravelly silty sandy clayey TOPSOIL with abundant roots and rootlets. Gravel consists of fine to medium subrounded to rounded flint and fine to coarse subangular limestone and sandstone.		1	
								Orange brown slightly gravelly slightly clayey silty fine to medium SAND with abundant roots and rootlets. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)			
					1.10			Orange brown slightly silty slightly clayey very sandy subrounded to rounded fine to coarse flint GRAVEL. (THANET FORMATION)			
					1.30			Orange brown slightly clayey silty gravelly fine to medium SAND with frequent clayey pockets. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)			
					1.75			Orange brown slightly gravelly slightly clayey silty fine to medium SAND with occasional clayey pockets. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)			
					2.10			Light yellowish brown slightly clayey slightly gravelly silty fine SAND. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)			
					3.00			End of Trial Pit at 3.000m		3	
										4	
										5	
Dimensions		Trench Support and Comment				Pumping Data					
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks		Date	Rate	Remarks			
2.30	0.45	Instability within sidewalls.	None.								
Remarks											
Trial pit dry upon completion. UXO engineer supervised excavation.											



Trial Pit Log

Project Name: Northfleet School for Girls		Client: Labopsort Ltd.		Date: 28/04/2025	
Location:		Contractor:			
Project No. : 27677		Crew Name:		Equipment: 3T Excavator.	
Location Number SA02	Location Type TP	Level	Logged By LR	Scale 1:25	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
					0.20			Grass over brown slightly clayey slightly gravelly silty sandy clayey TOPSOIL with abundant roots and rootlets. Gravel consists of fine to medium subrounded to rounded flint and fine to coarse subangular sandstone.	
					1.00			Orange brown slightly gravelly slightly clayey silty fine to medium SAND with abundant roots and rootlets. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)	1
					1.20			Orange brown slightly silty slightly clayey very sandy subrounded to rounded fine to coarse flint GRAVEL. (THANET FORMATION)	
					2.00			Orange brown slightly clayey silty gravelly fine to medium SAND with frequent clayey pockets. Gravel consists of fine to medium subrounded to rounded flint. (THANET FORMATION)	
								End of Trial Pit at 2.000m	2
									3
									4
									5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks
1.50	0.45	Instability within sidewalls.	None.				

Remarks	
Trial pit dry upon completion. UXO engineer supervised excavation.	



Trial pit dry upon completion. UXO engineer supervised excavation.



Remarks
Trial pit dry upon completion. UXO engineer supervised excavation.

Photographic Records – Trial Pits

Project: Northfleet School for Girls

Project Ref: 27677

Date: 28/04/25

Revision: 0



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W: www.jppuk.net

SA01 – Ground level to 3.00m bgl.



Photographic Records – Trial Pits

Project: Northfleet School for Girls

Project Ref: 27677

Date: 28/04/25

Revision: 0



T: 01604 781811

E: mail@jppuk.net

W: www.jppuk.net

SA02 – Ground level to 2.00m bgl.



Photographic Records – Trial Pits

Project: Northfleet School for Girls

Project Ref: 27677

Date: 28/04/25

Revision: 0



T: 01604 781811

E: mail@jppuk.net

W: www.jppuk.net

SA03 – Ground level to 1.50m bgl.



Photographic Records – Trial Pits

Project: Northfleet School for Girls

Project Ref: 27677

Date: 28/04/25

Revision: 0



T: 01604 781811

E: mail@jppuk.net

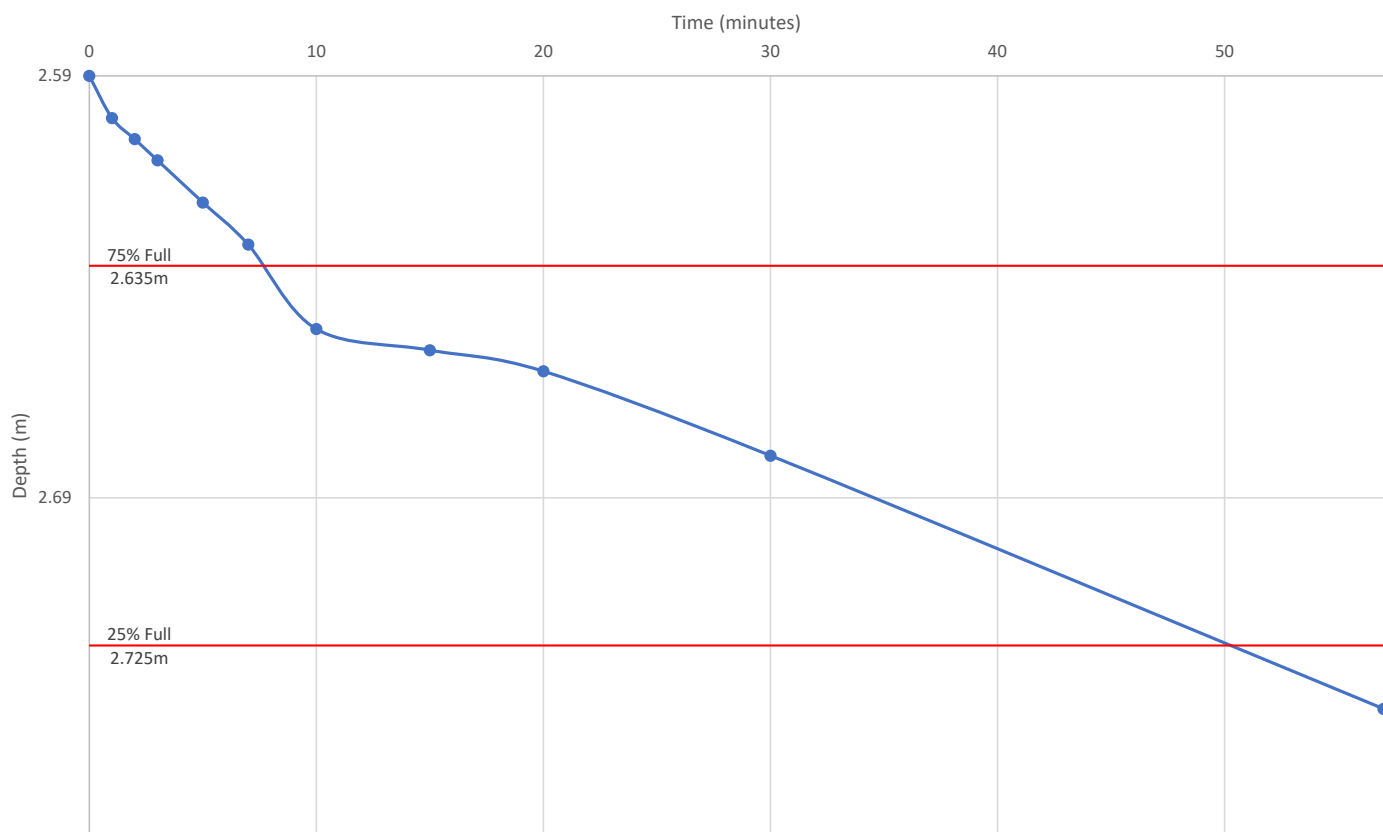
W: www.jppuk.net

SA04 – Ground level to 2.45m bgl.



BRE 365 Infiltration Test Result		 T: 01604 781811 E: mail@jppuk.net W: www.jppuk.net
Project Name:	Northfleet School for Girls	
Project No.	27677	
Client	Labosport	
Date:	28 April 2025	
Revision:	0	

SA01 - Cycle 1



Trial pit width = 0.45m
Trial pit length = 2.3m
Trial pit depth = 2.77m

Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

= 2.39E-05 m/s

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth

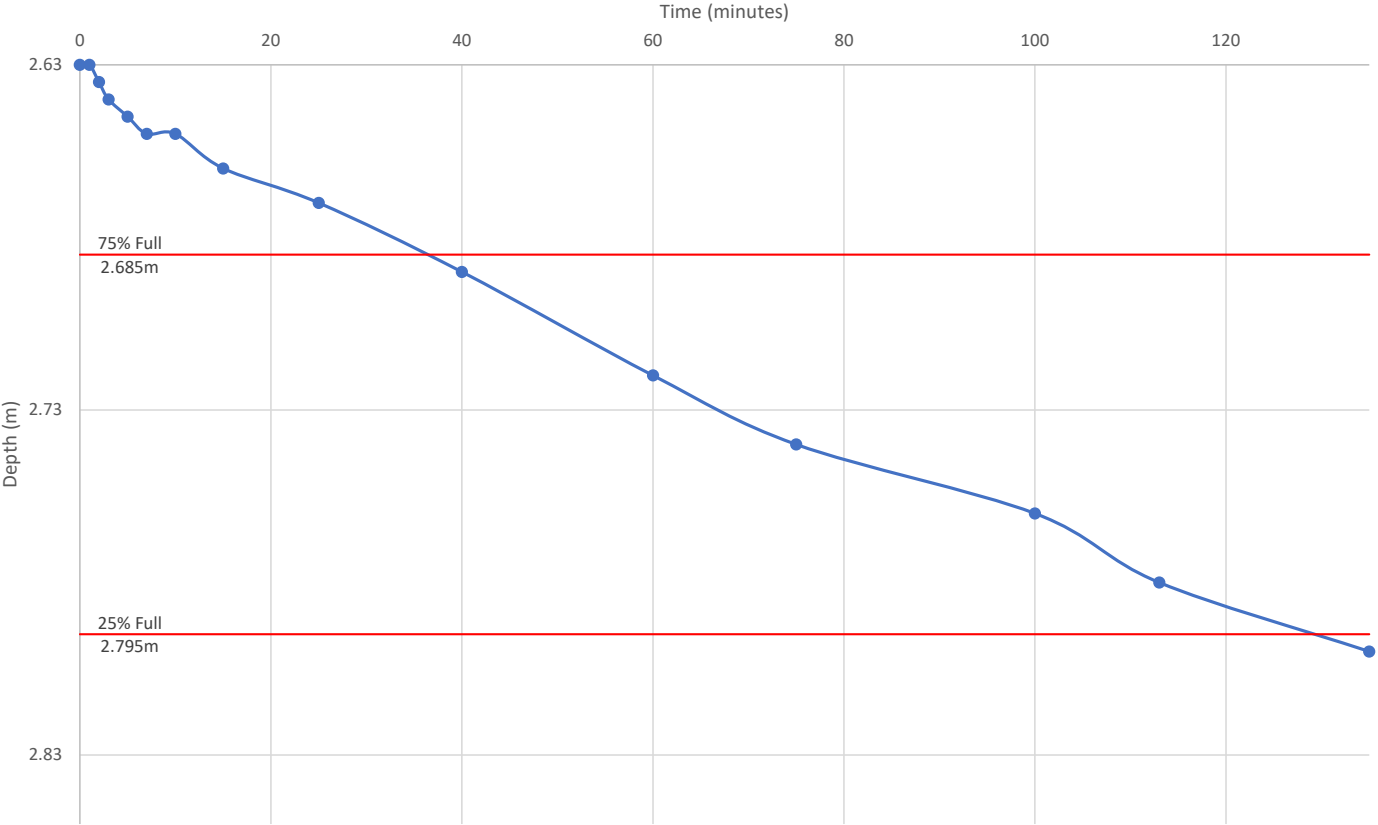
BRE 365 Infiltration Test Result

Project Name:	Northfleet School for Girls
Project No.	27677
Client	Labosport
Date:	28 April 2025
Revision:	0



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SA01 - Cycle 2



Trial pit width = 0.45m
Trial pit length = 2.3m
Trial pit depth = 2.85m

Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

= 1.24E-05 m/s

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth

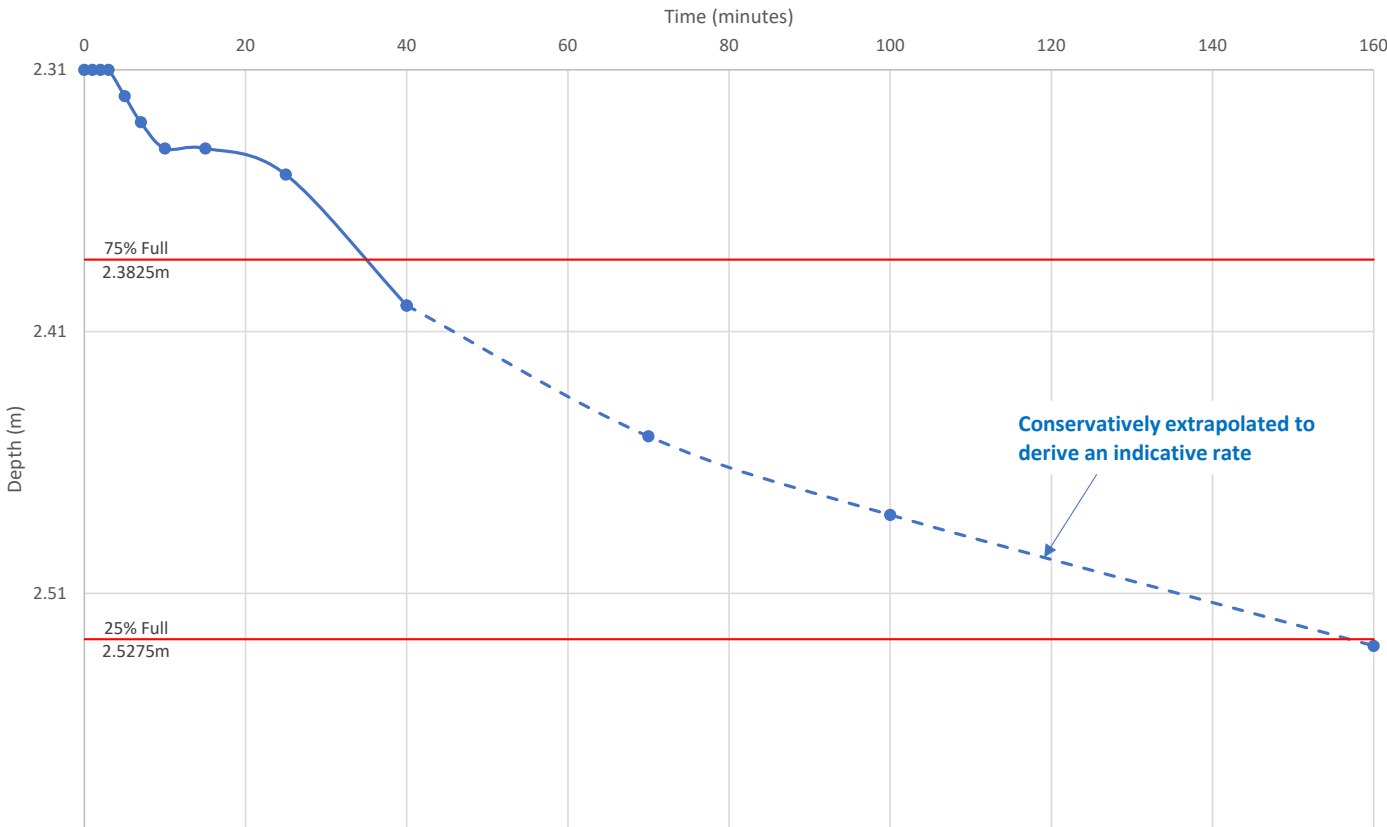
BRE 365 Infiltration Test Result

Project Name:	Northfleet School for Girls
Project No.	27677
Client	Labosport
Date:	28 April 2025
Revision:	0

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SA01 - Cycle 3



Trial pit width = 0.45m
Trial pit length = 2.3m
Trial pit depth = 2.6m

Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

= 1.11E-05 m/s

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth

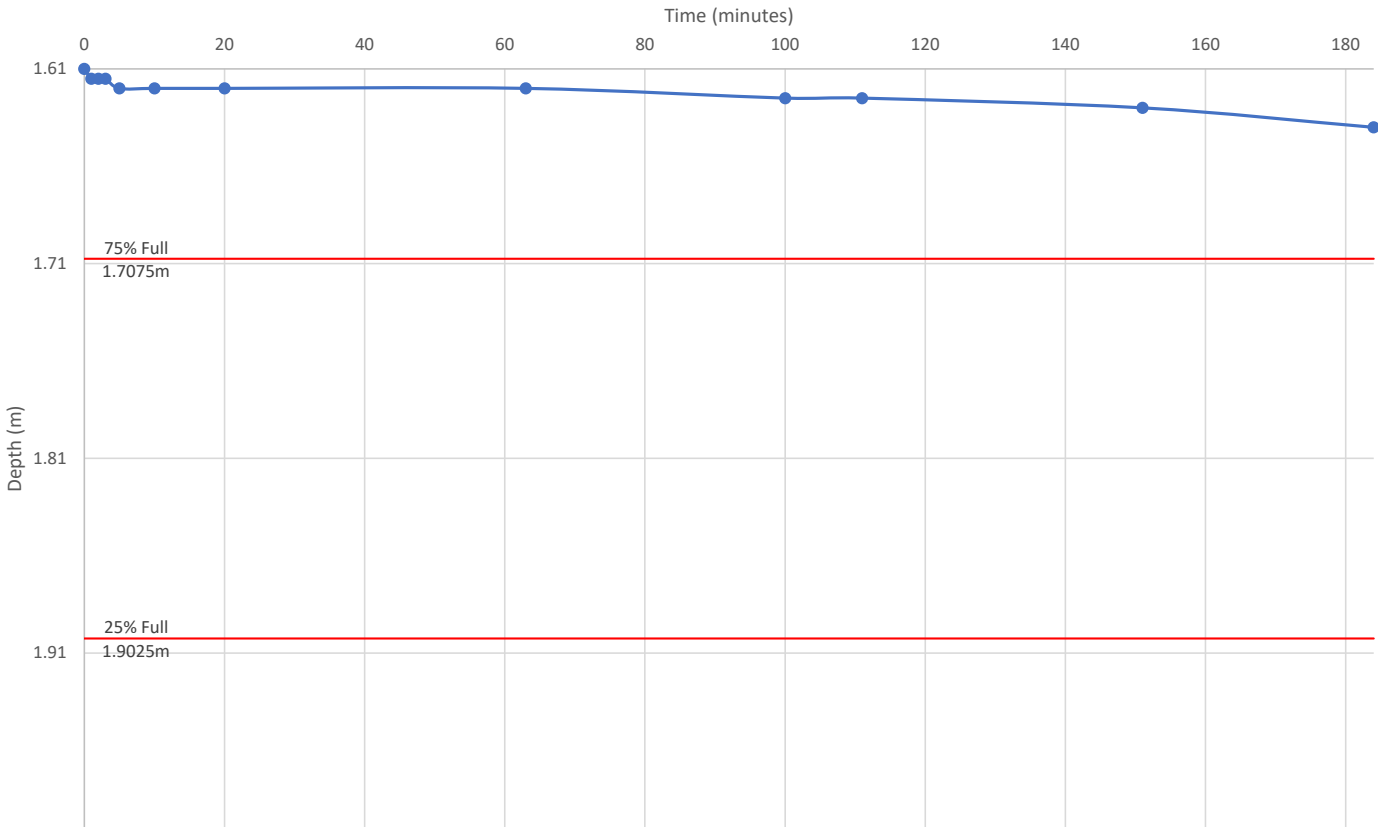
BRE 365 Infiltration Test Result

Project Name:	Northfleet School for Girls
Project No.	27677
Client	Labosport
Date:	28 April 2025
Revision:	0



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SA02 - Cycle 1



Trial pit width = 0.45m
Trial pit length = 1.5m
Trial pit depth = 2m

Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

Insufficient infiltration to derive a rate

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth

BRE 365 Infiltration Test Result

Project Name: Northfleet School for Girls

Project No. 27677

Client Labosport

Date: 28 April 2025

Revision: 0

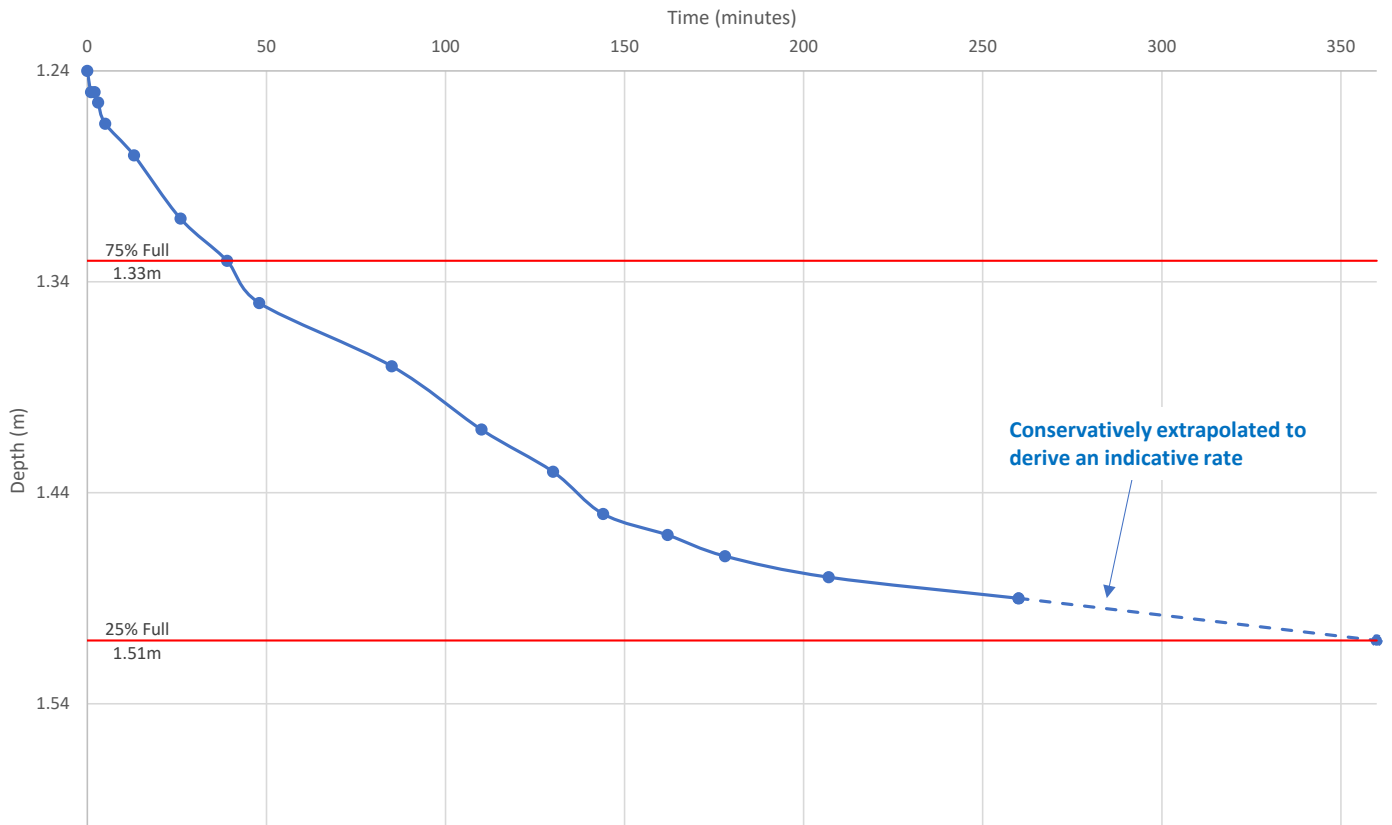


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SA03 - Cycle 1



Trial pit width = 0.45m

Trial pit length = 1.3m

Trial pit depth = 1.6m

$$\text{Soil infiltration rate } f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$$

$$= 4.50E-06 \text{ m/s}$$

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth

a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area

t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth

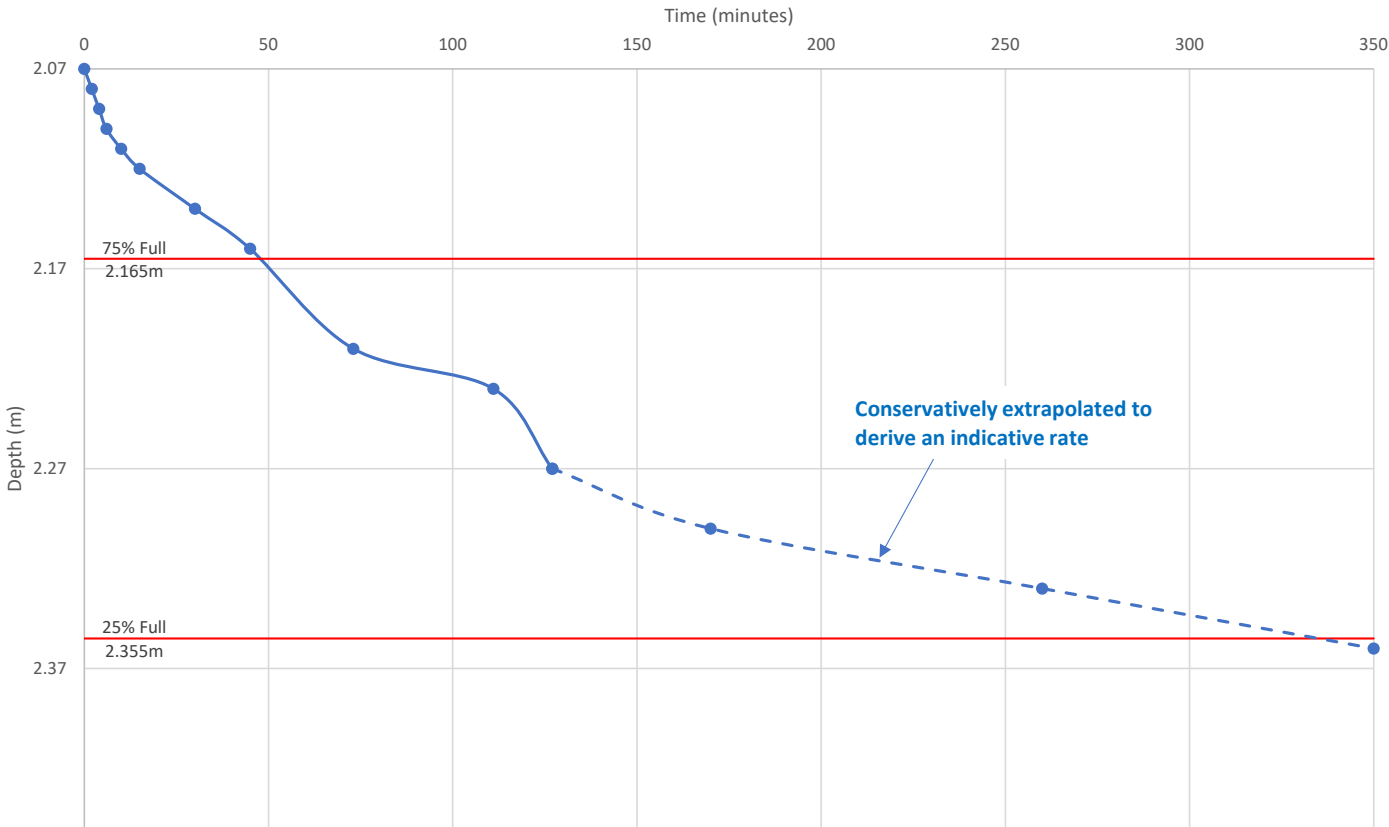
BRE 365 Infiltration Test Result

Project Name:	Northfleet School for Girls
Project No.	27677
Client	Labosport
Date:	28 April 2025
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SA04 - Cycle 1



Trial pit width = 0.45m
Trial pit length = 1.6m
Trial pit depth = 2.45m

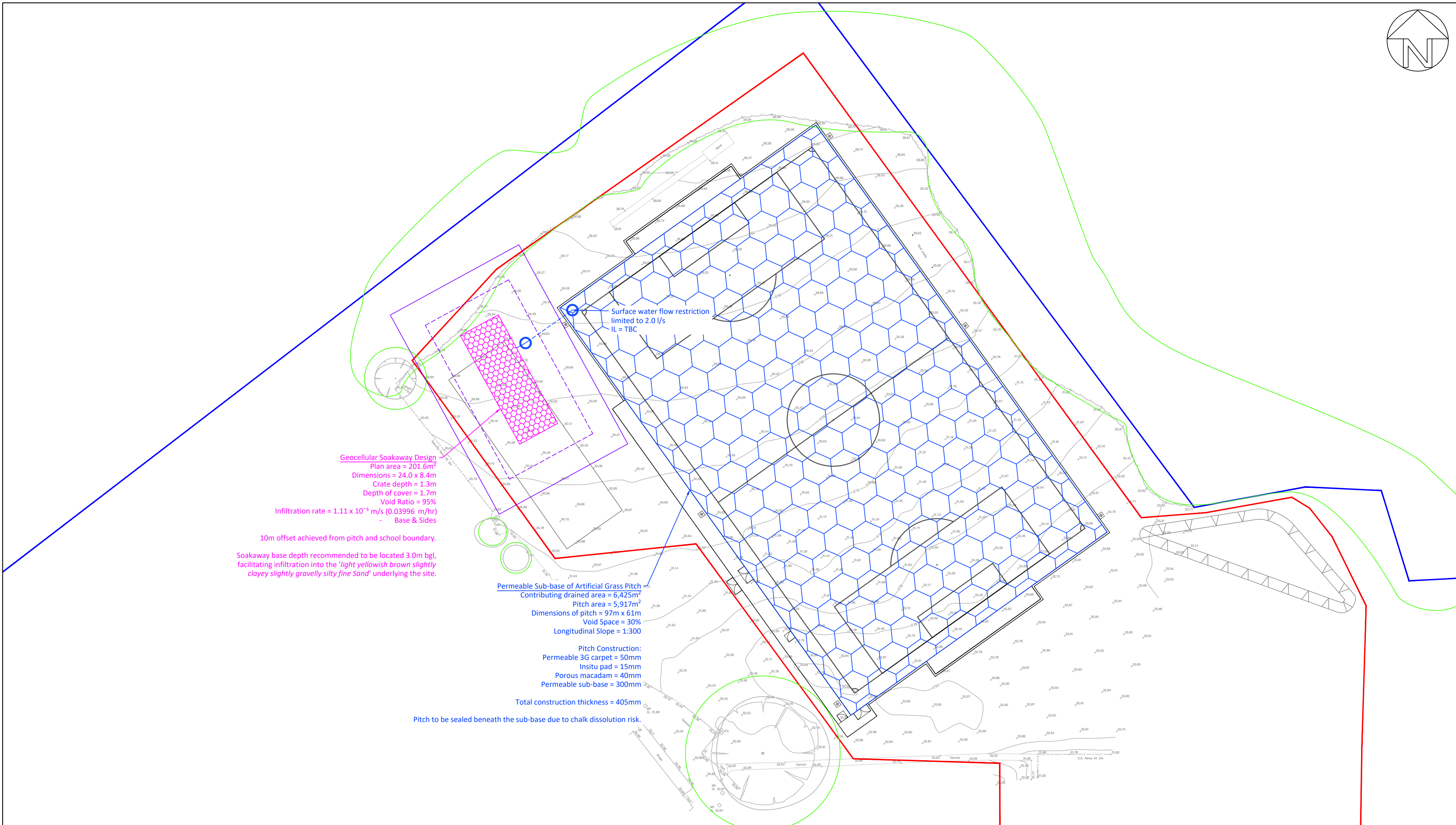
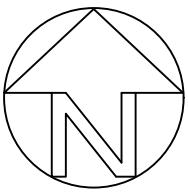
Soil infiltration rate $f = \frac{V_{p75-25}}{a_{s50} \times t_{p75-25}}$

= 5.28E-06 m/s

V_{p75-25} : the effective storage volume of water in the soakage trial pit between 75% and 25% effective storage depth
 a_{s50} : the internal surface area of the soakage trial pit up to 50% effective storage depth and including base area
 t_{p75-25} : the time for the water level to fall from 75% to 25% effective storage depth



Appendix F
Proposed Drainage Strategy
JPP Consulting drawing no. 28616-FRA03A



Geocellular Soakaway Design
Plan area = 201.6m²
Dimensions = 24.0 x 8.4m
Crate depth = 1.3m
Depth of cover = 1.7m
Void Ratio = 95%
Infiltration rate = 1.11 x 10⁻⁵ m/s (0.03996 m/hr)
- Base & Sides

10m offset achieved from pitch and school boundary.
Soakaway base depth recommended to be located 3.0m bgl,
facilitating infiltration into the 'light yellowish brown slightly
clayey slightly gravelly silty fine Sand' underlying the site.

Permeable Sub-base of Artificial Grass Pitch
Contributing drained area = 6,425m²
Pitch area = 5,917m²
Dimensions of pitch = 97m x 61m
Void Space = 30%
Longitudinal Slope = 1:300

Pitch Construction:
Permeable 3G carpet = 50mm
Insitu pad = 15mm
Porous macadam = 40mm
Permeable sub-base = 300mm

Total construction thickness = 405mm

Pitch to be sealed beneath the sub-base due to chalk dissolution risk.

General Notes

- All dimensions are in metres unless otherwise stated.
- All levels are in metres.
- This drawing is to be read in conjunction with all relevant Engineers and Architect's drawings, Specifications, Reports and Engineering Details.
- Do not scale from this drawing.

Drawing Key

- Site Boundary
- Ownership Boundary
- Proposed Surface Water Drainage
- Extent of permeable sub-base = 5,917m²
- Proposed Geocellular Soakaway
- Soakaway Offset (5m)
- Soakaway Offset (10m)
- Root Protection Area

This drawing has been based/developed using the following external drawings

Type	Company	Drawing date	Drawing title	Date received	Rev
Site Layout	Labosport Ltd	17.05.2024	Proposed Plan	11.07.2024	-
Topographical Survey	JPP Surveying	09.02.2024	Topographical Survey	11.07.2024	-
Tree Survey	-	-	C180683-01-01	04.07.2025	-

Rev A Drainage strategy revised based on formal soakaway testing. By TMK Checked KER 21.07.2025

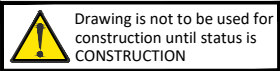
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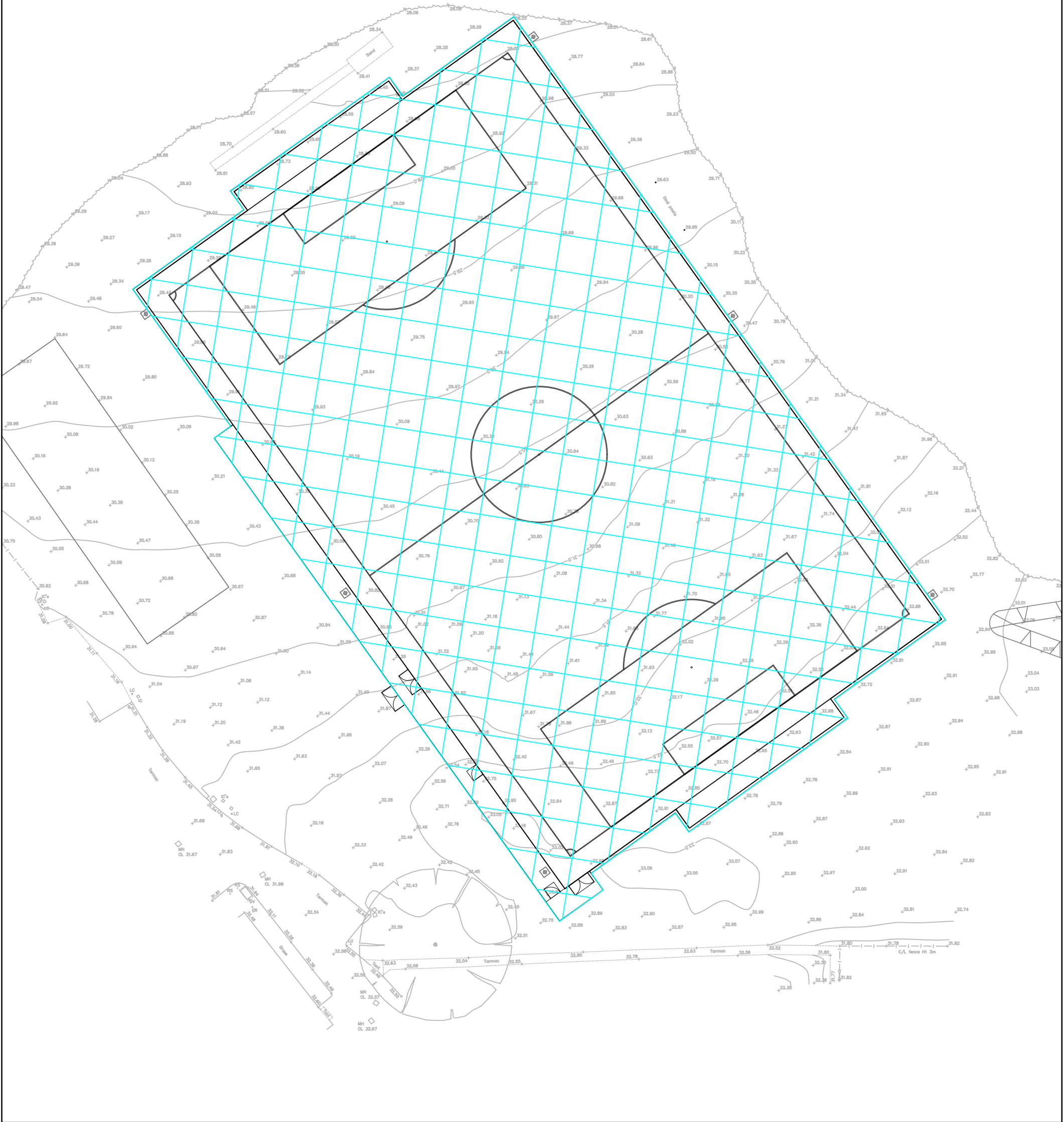
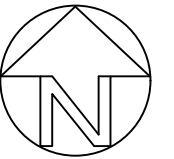
Client Labosport Ltd
Project Proposed AGP
Northfleet School for Girls
Hall Road, Northfleet, Gravesend
Title Proposed Drainage Strategy

Scale at A2 1:500	Drawn by JPB	Checked by KER	Date 29.08.2024
Status FOR PLANNING	Project ref 28616	Drawing no. FRA03	Revision A





Appendix G
Proposed Drained Area
JPP Consulting drawing no. 28616-FRA02



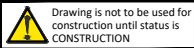
General Notes

- 1. All dimensions are in metres unless otherwise stated.
- 2. All levels are in metres.
- 3. This drawing is to be read in conjunction with all relevant Engineers and Architect's drawings, Specifications, Reports and Engineering Details.
- 4. Do not scale from this drawing.

Drawing Key

 Proposed Drained Area = 6,425m²

This drawing has been based/developed using the following external drawings					
Type	Company	Drawing date	Drawing title	Date received	Rev
Site Layout	Labosport Ltd	17.05.2024	Proposed Plan	11.07.2024	-
Topographical Survey	JPP Surveying	09.02.2024	Topographical Survey	11.07.2024	-





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- Infrastructure Design
- Structural Engineering
- Development Planning
- Professional Advice
- Geotechnical & Environmental
- Surveying

Client Labosport Ltd

Project Proposed AGP
Northfleet School for Girls
Hall Road, Northfleet, Gravesend

Title Proposed Drained Area

Scale at A3 1:500	Drawn by JPB	Checked by KER	Date 21.08.2024
Status FOR PLANNING	Project ref 28616	Drawing no. FRA02	Revision -



Appendix H

Soakaway Calculation (Cascade)

JPP Consulting Ltd			Page 1		
4, Ironstone Way Brixworth Northampton, NN3 9UD		Proposed AGP Northfleet School for Girls Cascade Calculation			
Date 17/07/2025 10:49 File 28616 - Cascade (8.4 x ...		Designed by TMK Checked by KER			
Innovyze		Source Control 2020.1.3			
<u>Cascade Summary of Results for 28616 - Attenuation Cellular Storage 1.3m</u> <u>(Cascade) Option 2.SRCX</u>					
Upstream Structures		Outflow To Overflow To			
28616 - Pitch Calcs.SRCX		(None)	(None)		
Half Drain Time : 1413 minutes.					
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Volume (m³)	Status
15 min Summer	26.737	0.337	1.2	64.6	O K
30 min Summer	26.744	0.344	1.2	65.8	O K
60 min Summer	26.995	0.595	1.3	114.0	O K
120 min Summer	27.012	0.612	1.3	117.2	O K
180 min Summer	27.020	0.620	1.3	118.8	O K
240 min Summer	27.025	0.625	1.3	119.7	O K
360 min Summer	27.030	0.630	1.3	120.7	O K
480 min Summer	27.032	0.632	1.3	121.1	O K
600 min Summer	27.032	0.632	1.3	121.0	O K
720 min Summer	27.030	0.630	1.3	120.7	O K
960 min Summer	27.032	0.632	1.3	121.1	O K
1440 min Summer	27.026	0.626	1.3	119.9	O K
2160 min Summer	27.449	1.049	1.5	200.9	O K
2880 min Summer	27.450	1.050	1.5	201.0	O K
4320 min Summer	27.374	0.974	1.5	186.5	O K
5760 min Summer	27.567	1.167	1.5	223.5	O K
7200 min Summer	27.549	1.149	1.5	220.0	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Time-Peak (mins)		
15 min Summer	263.189	0.0	1440		
30 min Summer	146.195	0.0	1440		
60 min Summer	81.208	0.0	2880		
120 min Summer	45.109	0.0	2880		
180 min Summer	31.982	0.0	2880		
240 min Summer	25.057	0.0	2880		
360 min Summer	17.765	0.0	2880		
480 min Summer	13.919	0.0	2880		
600 min Summer	11.518	0.0	2880		
720 min Summer	9.868	0.0	2880		
960 min Summer	8.061	0.0	2880		
1440 min Summer	6.061	0.0	2880		
2160 min Summer	4.557	0.0	5760		
2880 min Summer	3.723	0.0	5760		
4320 min Summer	2.609	0.0	5760		
5760 min Summer	2.027	0.0	8512		
7200 min Summer	1.666	0.0	9128		
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JPP Consulting Ltd				Page 2	
4, Ironstone Way Brixworth Northampton, NN3 9UD		Proposed AGP Northfleet School for Girls Cascade Calculation			
Date 17/07/2025 10:49 File 28616 - Cascade (8.4 x ...		Designed by TMK Checked by KER			
Innovyze		Source Control 2020.1.3			
<u>Cascade Summary of Results for 28616 - Attenuation_Cellular Storage_1.3m</u> <u>(Cascade)_Option 2.SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Volume (m³)	Status
8640 min Summer	27.531	1.131	1.5	216.6	O K
10080 min Summer	27.510	1.110	1.5	212.7	O K
15 min Winter	26.745	0.345	1.2	66.1	O K
30 min Winter	26.752	0.352	1.2	67.4	O K
60 min Winter	27.016	0.616	1.3	117.9	O K
120 min Winter	27.032	0.632	1.3	121.0	O K
180 min Winter	27.040	0.640	1.3	122.5	O K
240 min Winter	27.045	0.645	1.4	123.5	O K
360 min Winter	27.050	0.650	1.4	124.5	O K
480 min Winter	27.052	0.652	1.4	124.8	O K
600 min Winter	27.051	0.651	1.4	124.7	O K
720 min Winter	27.050	0.650	1.4	124.5	O K
960 min Winter	27.052	0.652	1.4	124.8	O K
1440 min Winter	27.045	0.645	1.4	123.5	O K
2160 min Winter	27.493	1.093	1.5	209.4	O K
2880 min Winter	27.493	1.093	1.5	209.3	O K
4320 min Winter	27.430	1.030	1.5	197.3	O K
5760 min Winter	27.698	1.298	1.6	248.6	O K
7200 min Winter	27.682	1.282	1.6	245.5	O K
8640 min Winter	27.663	1.263	1.6	241.8	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Time-Peak (mins)		
8640 min Summer	1.420	0.0	9736		
10080 min Summer	1.241	0.0	10344		
15 min Winter	263.189	0.0	1440		
30 min Winter	146.195	0.0	1440		
60 min Winter	81.208	0.0	2880		
120 min Winter	45.109	0.0	2880		
180 min Winter	31.982	0.0	2880		
240 min Winter	25.057	0.0	2880		
360 min Winter	17.765	0.0	2880		
480 min Winter	13.919	0.0	2880		
600 min Winter	11.518	0.0	2880		
720 min Winter	9.868	0.0	2880		
960 min Winter	8.061	0.0	2880		
1440 min Winter	6.061	0.0	2880		
2160 min Winter	4.557	0.0	5760		
2880 min Winter	3.723	0.0	5760		
4320 min Winter	2.609	0.0	5760		
5760 min Winter	2.027	0.0	9248		
7200 min Winter	1.666	0.0	9816		
8640 min Winter	1.420	0.0	10376		
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JPP Consulting Ltd			Page 3		
4, Ironstone Way Brixworth Northampton, NN3 9UD		Proposed AGP Northfleet School for Girls Cascade Calculation			
Date 17/07/2025 10:49 File 28616 - Cascade (8.4 x ...		Designed by TMK Checked by KER			
Innovyze		Source Control 2020.1.3			
<u>Cascade Summary of Results for 28616 - Attenuation_Cellular Storage_1.3m</u> <u>(Cascade)_Option 2.SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Volume (m³)	Status
10080 min Winter	27.641	1.241	1.6	237.7	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Time-Peak (mins)		
10080 min Winter	1.241	0.0	10944		
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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls Cascade Calculation	
Date 17/07/2025 10:49 File 28616 - Cascade (8.4 x ...	Designed by TMK Checked by KER	
Innovyze	Source Control 2020.1.3	

Cascade Rainfall Details for 28616 - Attenuation_Cellular Storage_1.3m
(Cascade)_Option 2.SRCX

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location	562900 172050 TQ 62900 72050
C (1km)	-0.027
D1 (1km)	0.276
D2 (1km)	0.421
D3 (1km)	0.247
E (1km)	0.324
F (1km)	2.535
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+45

Time Area Diagram

Total Area (ha) 0.000

Time (mins)	Area (ha)
From: 0	To: 4 0.000

JPP Consulting Ltd		Page 5
4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls Cascade Calculation	
Date 17/07/2025 10:49 File 28616 - Cascade (8.4 x ...	Designed by TMK Checked by KER	
Innovyze Source Control 2020.1.3		

Cascade Model Details for 28616 - Attenuation Cellular Storage 1.3m (Cascade) Option 2.SRCX

Storage is Online Cover Level (m) 29.400

Cellular Storage Structure

Invert Level (m) 26.400 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.03996 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.03996

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	201.6	201.6	1.400	0.0	285.8
1.300	201.6	285.8			

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Appendix I
Attenuation Calculations: 1 in 30 year

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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 30 yr, 2.0 l/s	
Date 17/07/2025 10:27 File 28616 - PITCH CALCS.SRCX	Designed by TMK Checked by KER	
Innovyze	Source Control 2020.1.3	

Summary of Results for 30 year Return Period

Half Drain Time : 1069 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	29.853	0.200	0.0	1.8	1.8	109.9	Flood Risk
30 min Summer	29.869	0.216	0.0	1.8	1.8	127.9	Flood Risk
60 min Summer	29.885	0.232	0.0	1.9	1.9	147.1	Flood Risk
120 min Summer	29.899	0.246	0.0	1.9	1.9	166.6	Flood Risk
180 min Summer	29.907	0.254	0.0	1.9	1.9	177.4	Flood Risk
240 min Summer	29.912	0.259	0.0	1.9	1.9	184.0	Flood Risk
360 min Summer	29.917	0.264	0.0	1.9	1.9	191.2	Flood Risk
480 min Summer	29.919	0.266	0.0	1.9	1.9	193.7	Flood Risk
600 min Summer	29.919	0.266	0.0	1.9	1.9	193.7	Flood Risk
720 min Summer	29.917	0.264	0.0	1.9	1.9	191.9	Flood Risk
960 min Summer	29.921	0.268	0.0	1.9	1.9	197.9	Flood Risk
1440 min Summer	29.925	0.272	0.0	1.9	1.9	203.3	Flood Risk
2160 min Summer	29.925	0.272	0.0	1.9	1.9	203.0	Flood Risk
2880 min Summer	29.923	0.270	0.0	1.9	1.9	199.8	Flood Risk
4320 min Summer	29.899	0.246	0.0	1.9	1.9	166.7	Flood Risk
5760 min Summer	29.877	0.224	0.0	1.9	1.9	137.7	Flood Risk
7200 min Summer	29.855	0.202	0.0	1.8	1.8	112.0	Flood Risk
8640 min Summer	29.834	0.181	0.0	1.8	1.8	89.5	Flood Risk
10080 min Summer	29.813	0.160	0.0	1.8	1.8	70.0	Flood Risk
15 min Winter	29.868	0.215	0.0	1.8	1.8	126.8	Flood Risk

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	116.964	0.0	111.2	19
30 min Summer	66.466	0.0	130.3	34
60 min Summer	37.770	0.0	151.9	64
120 min Summer	21.463	0.0	176.0	124
180 min Summer	15.421	0.0	191.2	182
240 min Summer	12.197	0.0	202.6	242
360 min Summer	8.763	0.0	219.6	362
480 min Summer	6.931	0.0	231.9	482
600 min Summer	5.778	0.0	241.7	600
720 min Summer	4.980	0.0	249.6	720
960 min Summer	4.106	0.0	275.4	840
1440 min Summer	3.129	0.0	279.1	1110
2160 min Summer	2.384	0.0	357.9	1516
2880 min Summer	1.966	0.0	390.1	1932
4320 min Summer	1.396	0.0	402.1	2728
5760 min Summer	1.095	0.0	406.8	3520
7200 min Summer	0.907	0.0	406.5	4320
8640 min Summer	0.778	0.0	404.3	5016
10080 min Summer	0.683	0.0	399.0	5752
15 min Winter	116.964	0.0	128.1	19

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Page 2

4, Ironstone Way
Brixworth
Northampton, NN3 9UD

Date 17/07/2025 10:27
File 28616 - PITCH CALCS.SRCX

Innovyze

Proposed AGP
Northfleet School for Girls
FEH, 1 in 30 yr, 2.0 l/s

Designed by TMK
Checked by KER

Source Control 2020.1.3

Micro
Drainage

Summary of Results for 30 year Return Period

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
30 min Winter	29.884	0.231	0.0	1.9	1.9	147.1	Flood Risk
60 min Winter	29.901	0.248	0.0	1.9	1.9	168.8	Flood Risk
120 min Winter	29.917	0.264	0.0	1.9	1.9	191.2	Flood Risk
180 min Winter	29.925	0.272	0.0	1.9	1.9	203.7	Flood Risk
240 min Winter	29.931	0.278	0.0	1.9	1.9	211.6	Flood Risk
360 min Winter	29.937	0.284	0.0	1.9	1.9	220.9	Flood Risk
480 min Winter	29.939	0.286	0.0	1.9	1.9	225.0	Flood Risk
600 min Winter	29.940	0.287	0.0	1.9	1.9	226.2	Flood Risk
720 min Winter	29.940	0.287	0.0	1.9	1.9	225.5	Flood Risk
960 min Winter	29.945	0.292	0.0	1.9	1.9	234.3	Flood Risk
1440 min Winter	29.948	0.295	0.0	1.9	1.9	239.3	Flood Risk
2160 min Winter	29.948	0.295	0.0	1.9	1.9	238.2	Flood Risk
2880 min Winter	29.943	0.290	0.0	1.9	1.9	231.2	Flood Risk
4320 min Winter	29.911	0.258	0.0	1.9	1.9	182.8	Flood Risk
5760 min Winter	29.879	0.226	0.0	1.9	1.9	139.7	Flood Risk
7200 min Winter	29.846	0.193	0.0	1.8	1.8	102.3	Flood Risk
8640 min Winter	29.813	0.160	0.0	1.8	1.8	70.6	Flood Risk
10080 min Winter	29.780	0.127	0.0	1.8	1.8	44.5	Flood Risk

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
30 min Winter	66.466	0.0	149.5	33
60 min Winter	37.770	0.0	173.6	62
120 min Winter	21.463	0.0	200.8	122
180 min Winter	15.421	0.0	218.0	180
240 min Winter	12.197	0.0	231.0	238
360 min Winter	8.763	0.0	249.9	356
480 min Winter	6.931	0.0	264.0	470
600 min Winter	5.778	0.0	275.1	584
720 min Winter	4.980	0.0	284.2	694
960 min Winter	4.106	0.0	293.6	914
1440 min Winter	3.129	0.0	286.1	1196
2160 min Winter	2.384	0.0	407.5	1644
2880 min Winter	1.966	0.0	444.9	2104
4320 min Winter	1.396	0.0	460.8	2984
5760 min Winter	1.095	0.0	468.2	3808
7200 min Winter	0.907	0.0	471.0	4544
8640 min Winter	0.778	0.0	471.4	5272
10080 min Winter	0.683	0.0	468.7	5952

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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 30 yr, 2.0 l/s	
Date 17/07/2025 10:27 File 28616 - PITCH CALCS.SRCX	Designed by TMK Checked by KER	
Innovyze Source Control 2020.1.3		

Rainfall Details

Rainfall Model	FEH
Return Period (years)	30
FEH Rainfall Version	1999
Site Location	562900 172050 TQ 62900 72050
C (1km)	-0.027
D1 (1km)	0.276
D2 (1km)	0.421
D3 (1km)	0.247
E (1km)	0.324
F (1km)	2.535
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+0

Time Area Diagram

Total Area (ha) 0.643

Time (mins)	Area
From: To:	(ha)
0	4 0.643

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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 30 yr, 2.0 l/s	
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Innovyze Source Control 2020.1.3		

Model Details

Storage is Online Cover Level (m) 30.058

Porous Car Park Structure

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	61.0
Membrane Percolation (mm/hr)	1000	Length (m)	97.0
Max Percolation (l/s)	1643.6	Slope (1:X)	300.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	29.653	Membrane Depth (m)	0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0064-2000-1200-2000
Design Head (m)	1.200
Design Flow (l/s)	2.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	64
Invert Level (m)	28.858
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	2.0
Flush-Flo™	0.282	1.8
Kick-Flo®	0.573	1.4
Mean Flow over Head Range	-	1.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.5	1.200	2.0	3.000	3.0	7.000	4.5
0.200	1.7	1.400	2.1	3.500	3.3	7.500	4.7
0.300	1.8	1.600	2.3	4.000	3.5	8.000	4.8
0.400	1.7	1.800	2.4	4.500	3.7	8.500	5.0
0.500	1.6	2.000	2.5	5.000	3.9	9.000	5.1
0.600	1.5	2.200	2.6	5.500	4.0	9.500	5.2
0.800	1.7	2.400	2.7	6.000	4.2		
1.000	1.8	2.600	2.8	6.500	4.4		

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Appendix J
Attenuation Calculations: 1 in 100 year

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4, Ironstone Way
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Proposed AGP
Northfleet School for Girls
FEH, 1 in 100 yr, 2.0 l/s

Designed by TMK
Checked by KER

Source Control 2020.1.3

Page 1



Summary of Results for 100 year Return Period

Half Drain Time : 1575 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	29.914	0.261	0.0	1.9	1.9	187.6	Flood Risk
30 min Summer	29.930	0.277	0.0	1.9	1.9	210.5	Flood Risk
60 min Summer	29.945	0.292	0.0	1.9	1.9	234.6	Flood Risk
120 min Summer	29.960	0.307	0.0	1.9	1.9	258.7	Flood Risk
180 min Summer	29.968	0.315	0.0	1.9	1.9	271.8	Flood Risk
240 min Summer	29.972	0.319	0.0	1.9	1.9	280.0	Flood Risk
360 min Summer	29.977	0.324	0.0	1.9	1.9	289.0	Flood Risk
480 min Summer	29.980	0.327	0.0	1.9	1.9	292.6	Flood Risk
600 min Summer	29.980	0.327	0.0	1.9	1.9	293.2	Flood Risk
720 min Summer	29.979	0.326	0.0	1.9	1.9	291.8	Flood Risk
960 min Summer	29.984	0.331	0.0	1.9	1.9	301.5	Flood Risk
1440 min Summer	29.988	0.335	0.0	1.9	1.9	307.1	Flood Risk
2160 min Summer	29.987	0.334	0.0	1.9	1.9	305.6	Flood Risk
2880 min Summer	29.984	0.331	0.0	1.9	1.9	300.8	Flood Risk
4320 min Summer	29.959	0.306	0.0	1.9	1.9	257.4	Flood Risk
5760 min Summer	29.937	0.284	0.0	1.9	1.9	220.9	Flood Risk
7200 min Summer	29.915	0.262	0.0	1.9	1.9	188.5	Flood Risk
8640 min Summer	29.894	0.241	0.0	1.9	1.9	159.7	Flood Risk
10080 min Summer	29.874	0.221	0.0	1.9	1.9	133.8	Flood Risk
15 min Winter	29.932	0.279	0.0	1.9	1.9	213.8	Flood Risk

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	181.510	0.0	157.6	19
30 min Summer	100.824	0.0	158.5	34
60 min Summer	56.005	0.0	239.8	64
120 min Summer	31.110	0.0	269.1	124
180 min Summer	22.056	0.0	287.4	184
240 min Summer	17.281	0.0	300.9	242
360 min Summer	12.252	0.0	311.1	362
480 min Summer	9.599	0.0	309.7	482
600 min Summer	7.944	0.0	307.9	602
720 min Summer	6.806	0.0	305.7	722
960 min Summer	5.559	0.0	303.0	960
1440 min Summer	4.180	0.0	295.6	1256
2160 min Summer	3.143	0.0	489.6	1648
2880 min Summer	2.567	0.0	529.3	2048
4320 min Summer	1.799	0.0	527.6	2852
5760 min Summer	1.398	0.0	546.8	3640
7200 min Summer	1.149	0.0	546.9	4464
8640 min Summer	0.979	0.0	543.9	5192
10080 min Summer	0.856	0.0	539.7	5952
15 min Winter	181.510	0.0	159.2	19

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Date 17/07/2025 10:26 File 28616 - PITCH CALCS.SRCX			Designed by TMK Checked by KER				
Innovyze			Source Control 2020.1.3				
<u>Summary of Results for 100 year Return Period</u>							
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
30 min Winter	29.948	0.295	0.0	1.9	1.9	239.6	Flood Risk
60 min Winter	29.965	0.312	0.0	1.9	1.9	266.7	Flood Risk
120 min Winter	29.981	0.328	0.0	1.9	1.9	294.5	Flood Risk
180 min Winter	29.989	0.336	0.0	1.9	1.9	309.8	Flood Risk
240 min Winter	29.995	0.342	0.0	1.9	1.9	319.6	Flood Risk
360 min Winter	30.001	0.348	0.0	2.0	2.0	331.1	Flood Risk
480 min Winter	30.004	0.351	0.0	2.0	2.0	336.5	Flood Risk
600 min Winter	30.005	0.352	0.0	2.0	2.0	338.5	Flood Risk
720 min Winter	30.005	0.352	0.0	2.0	2.0	338.2	Flood Risk
960 min Winter	30.013	0.360	0.0	2.0	2.0	352.3	Flood Risk
1440 min Winter	30.019	0.366	0.0	2.0	2.0	363.6	Flood Risk
2160 min Winter	30.018	0.365	0.0	2.0	2.0	360.6	Flood Risk
2880 min Winter	30.014	0.361	0.0	2.0	2.0	353.5	Flood Risk
4320 min Winter	29.981	0.328	0.0	1.9	1.9	294.4	Flood Risk
5760 min Winter	29.950	0.297	0.0	1.9	1.9	241.5	Flood Risk
7200 min Winter	29.919	0.266	0.0	1.9	1.9	194.3	Flood Risk
8640 min Winter	29.889	0.236	0.0	1.9	1.9	152.5	Flood Risk
10080 min Winter	29.858	0.205	0.0	1.8	1.8	115.8	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)			
30 min Winter	100.824	0.0	160.3	34			
60 min Winter	56.005	0.0	272.3	64			
120 min Winter	31.110	0.0	305.0	122			
180 min Winter	22.056	0.0	316.4	180			
240 min Winter	17.281	0.0	316.8	240			
360 min Winter	12.252	0.0	316.2	356			
480 min Winter	9.599	0.0	314.8	474			
600 min Winter	7.944	0.0	313.1	590			
720 min Winter	6.806	0.0	311.1	702			
960 min Winter	5.559	0.0	308.6	932			
1440 min Winter	4.180	0.0	301.6	1370			
2160 min Winter	3.143	0.0	555.0	1756			
2880 min Winter	2.567	0.0	594.5	2216			
4320 min Winter	1.799	0.0	551.8	3112			
5760 min Winter	1.398	0.0	625.1	3976			
7200 min Winter	1.149	0.0	628.2	4760			
8640 min Winter	0.979	0.0	627.8	5536			
10080 min Winter	0.856	0.0	625.1	6256			
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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 100 yr, 2.0 l/s	
Date 17/07/2025 10:26 File 28616 - PITCH CALCS.SRCX	Designed by TMK Checked by KER	
Innovyze Source Control 2020.1.3		

Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location	562900 172050 TQ 62900 72050
C (1km)	-0.027
D1 (1km)	0.276
D2 (1km)	0.421
D3 (1km)	0.247
E (1km)	0.324
F (1km)	2.535
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+0

Time Area Diagram

Total Area (ha) 0.643

Time (mins)	Area
From:	To: (ha)
0	4 0.643

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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 100 yr, 2.0 l/s	
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Innovyze Source Control 2020.1.3		

Model Details

Storage is Online Cover Level (m) 30.058

Porous Car Park Structure

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	61.0
Membrane Percolation (mm/hr)	1000	Length (m)	97.0
Max Percolation (l/s)	1643.6	Slope (1:X)	300.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	29.653	Membrane Depth (m)	0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0064-2000-1200-2000
Design Head (m)	1.200
Design Flow (l/s)	2.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	64
Invert Level (m)	28.858
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	2.0
Flush-Flo™	0.282	1.8
Kick-Flo®	0.573	1.4
Mean Flow over Head Range	-	1.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.5	1.200	2.0	3.000	3.0	7.000	4.5
0.200	1.7	1.400	2.1	3.500	3.3	7.500	4.7
0.300	1.8	1.600	2.3	4.000	3.5	8.000	4.8
0.400	1.7	1.800	2.4	4.500	3.7	8.500	5.0
0.500	1.6	2.000	2.5	5.000	3.9	9.000	5.1
0.600	1.5	2.200	2.6	5.500	4.0	9.500	5.2
0.800	1.7	2.400	2.7	6.000	4.2		
1.000	1.8	2.600	2.8	6.500	4.4		

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Appendix K
Attenuation Calculations: 1 in 100 year + 45% CC

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Date 17/07/2025 10:25 File 28616 - PITCH CALCS.SRCX			Designed by TMK Checked by KER																																																																																																																																																																																																																																																																																					
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<u>Summary of Results for 100 year Return Period (+45%)</u> Half Drain Time : 2587 minutes. <table><tr><th>Storm Event</th><th>Max Level (m)</th><th>Max Depth (m)</th><th>Max Infiltration (l/s)</th><th>Max Control (l/s)</th><th>Max E Outflow (l/s)</th><th>Max Volume (m³)</th><th>Status</th></tr><tr><td>15 min Summer</td><td>29.976</td><td>0.323</td><td>0.0</td><td>1.9</td><td>1.9</td><td>285.9</td><td>Flood Risk</td></tr><tr><td>30 min Summer</td><td>29.995</td><td>0.342</td><td>0.0</td><td>1.9</td><td>1.9</td><td>319.7</td><td>Flood Risk</td></tr><tr><td>60 min Summer</td><td>30.015</td><td>0.362</td><td>0.0</td><td>2.0</td><td>2.0</td><td>355.6</td><td>Flood Risk</td></tr><tr><td>120 min Summer</td><td>30.036</td><td>0.383</td><td>0.0</td><td>2.0</td><td>2.0</td><td>392.6</td><td>Flood Risk</td></tr><tr><td>180 min Summer</td><td>30.048</td><td>0.395</td><td>0.0</td><td>2.0</td><td>2.0</td><td>413.8</td><td>Flood Risk</td></tr><tr><td>240 min Summer</td><td>30.056</td><td>0.403</td><td>0.0</td><td>2.0</td><td>2.0</td><td>427.9</td><td>Flood Risk</td></tr><tr><td>360 min Summer</td><td>30.063</td><td>0.410</td><td>0.0</td><td>2.0</td><td>2.0</td><td>445.3</td><td>FLOOD</td></tr><tr><td>480 min Summer</td><td>30.066</td><td>0.413</td><td>0.0</td><td>2.0</td><td>2.0</td><td>455.2</td><td>FLOOD</td></tr><tr><td>600 min Summer</td><td>30.068</td><td>0.415</td><td>0.0</td><td>2.0</td><td>2.0</td><td>460.4</td><td>FLOOD</td></tr><tr><td>720 min Summer</td><td>30.069</td><td>0.416</td><td>0.0</td><td>2.0</td><td>2.0</td><td>462.8</td><td>FLOOD</td></tr><tr><td>960 min Summer</td><td>30.078</td><td>0.425</td><td>0.0</td><td>2.0</td><td>2.0</td><td>486.2</td><td>FLOOD</td></tr><tr><td>1440 min Summer</td><td>30.087</td><td>0.434</td><td>0.0</td><td>2.0</td><td>2.0</td><td>510.6</td><td>FLOOD</td></tr><tr><td>2160 min Summer</td><td>30.090</td><td>0.437</td><td>0.0</td><td>2.0</td><td>2.0</td><td>519.0</td><td>FLOOD</td></tr><tr><td>2880 min Summer</td><td>30.090</td><td>0.437</td><td>0.0</td><td>2.0</td><td>2.0</td><td>517.6</td><td>FLOOD</td></tr><tr><td>4320 min Summer</td><td>30.067</td><td>0.414</td><td>0.0</td><td>2.0</td><td>2.0</td><td>457.8</td><td>FLOOD</td></tr><tr><td>5760 min Summer</td><td>30.047</td><td>0.394</td><td>0.0</td><td>2.0</td><td>2.0</td><td>412.2</td><td>Flood Risk</td></tr><tr><td>7200 min Summer</td><td>30.024</td><td>0.371</td><td>0.0</td><td>2.0</td><td>2.0</td><td>372.4</td><td>Flood Risk</td></tr><tr><td>8640 min Summer</td><td>30.004</td><td>0.351</td><td>0.0</td><td>2.0</td><td>2.0</td><td>336.4</td><td>Flood Risk</td></tr><tr><td>10080 min Summer</td><td>29.985</td><td>0.332</td><td>0.0</td><td>1.9</td><td>1.9</td><td>303.2</td><td>Flood Risk</td></tr><tr><td>15 min Winter</td><td>29.997</td><td>0.344</td><td>0.0</td><td>2.0</td><td>2.0</td><td>324.0</td><td>Flood Risk</td></tr></table> <table><tr><th>Storm Event</th><th>Rain (mm/hr)</th><th>Flooded Volume (m³)</th><th>Discharge Volume (m³)</th><th>Time-Peak (mins)</th></tr><tr><td>15 min Summer</td><td>263.189</td><td>0.0</td><td>163.1</td><td>19</td></tr><tr><td>30 min Summer</td><td>146.195</td><td>0.0</td><td>164.1</td><td>34</td></tr><tr><td>60 min Summer</td><td>81.208</td><td>0.0</td><td>323.7</td><td>64</td></tr><tr><td>120 min Summer</td><td>45.109</td><td>0.0</td><td>326.5</td><td>124</td></tr><tr><td>180 min Summer</td><td>31.982</td><td>0.0</td><td>327.4</td><td>184</td></tr><tr><td>240 min Summer</td><td>25.057</td><td>0.0</td><td>327.7</td><td>244</td></tr><tr><td>360 min Summer</td><td>17.765</td><td>5.0</td><td>327.3</td><td>362</td></tr><tr><td>480 min Summer</td><td>13.919</td><td>8.8</td><td>326.2</td><td>482</td></tr><tr><td>600 min Summer</td><td>11.518</td><td>10.5</td><td>324.8</td><td>602</td></tr><tr><td>720 min Summer</td><td>9.868</td><td>11.2</td><td>323.0</td><td>722</td></tr><tr><td>960 min Summer</td><td>8.061</td><td>20.3</td><td>321.1</td><td>962</td></tr><tr><td>1440 min Summer</td><td>6.061</td><td>29.1</td><td>315.5</td><td>1440</td></tr><tr><td>2160 min Summer</td><td>4.557</td><td>32.7</td><td>636.1</td><td>1968</td></tr><tr><td>2880 min Summer</td><td>3.723</td><td>32.1</td><td>628.8</td><td>2332</td></tr><tr><td>4320 min Summer</td><td>2.609</td><td>9.6</td><td>588.1</td><td>3068</td></tr><tr><td>5760 min Summer</td><td>2.027</td><td>0.0</td><td>837.7</td><td>3864</td></tr><tr><td>7200 min Summer</td><td>1.666</td><td>0.0</td><td>846.5</td><td>4680</td></tr><tr><td>8640 min Summer</td><td>1.420</td><td>0.0</td><td>850.3</td><td>5448</td></tr><tr><td>10080 min Summer</td><td>1.241</td><td>0.0</td><td>851.0</td><td>6256</td></tr><tr><td>15 min Winter</td><td>263.189</td><td>0.0</td><td>164.8</td><td>19</td></tr></table>								Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status	15 min Summer	29.976	0.323	0.0	1.9	1.9	285.9	Flood Risk	30 min Summer	29.995	0.342	0.0	1.9	1.9	319.7	Flood Risk	60 min Summer	30.015	0.362	0.0	2.0	2.0	355.6	Flood Risk	120 min Summer	30.036	0.383	0.0	2.0	2.0	392.6	Flood Risk	180 min Summer	30.048	0.395	0.0	2.0	2.0	413.8	Flood Risk	240 min Summer	30.056	0.403	0.0	2.0	2.0	427.9	Flood Risk	360 min Summer	30.063	0.410	0.0	2.0	2.0	445.3	FLOOD	480 min Summer	30.066	0.413	0.0	2.0	2.0	455.2	FLOOD	600 min Summer	30.068	0.415	0.0	2.0	2.0	460.4	FLOOD	720 min Summer	30.069	0.416	0.0	2.0	2.0	462.8	FLOOD	960 min Summer	30.078	0.425	0.0	2.0	2.0	486.2	FLOOD	1440 min Summer	30.087	0.434	0.0	2.0	2.0	510.6	FLOOD	2160 min Summer	30.090	0.437	0.0	2.0	2.0	519.0	FLOOD	2880 min Summer	30.090	0.437	0.0	2.0	2.0	517.6	FLOOD	4320 min Summer	30.067	0.414	0.0	2.0	2.0	457.8	FLOOD	5760 min Summer	30.047	0.394	0.0	2.0	2.0	412.2	Flood Risk	7200 min Summer	30.024	0.371	0.0	2.0	2.0	372.4	Flood Risk	8640 min Summer	30.004	0.351	0.0	2.0	2.0	336.4	Flood Risk	10080 min Summer	29.985	0.332	0.0	1.9	1.9	303.2	Flood Risk	15 min Winter	29.997	0.344	0.0	2.0	2.0	324.0	Flood Risk	Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	15 min Summer	263.189	0.0	163.1	19	30 min Summer	146.195	0.0	164.1	34	60 min Summer	81.208	0.0	323.7	64	120 min Summer	45.109	0.0	326.5	124	180 min Summer	31.982	0.0	327.4	184	240 min Summer	25.057	0.0	327.7	244	360 min Summer	17.765	5.0	327.3	362	480 min Summer	13.919	8.8	326.2	482	600 min Summer	11.518	10.5	324.8	602	720 min Summer	9.868	11.2	323.0	722	960 min Summer	8.061	20.3	321.1	962	1440 min Summer	6.061	29.1	315.5	1440	2160 min Summer	4.557	32.7	636.1	1968	2880 min Summer	3.723	32.1	628.8	2332	4320 min Summer	2.609	9.6	588.1	3068	5760 min Summer	2.027	0.0	837.7	3864	7200 min Summer	1.666	0.0	846.5	4680	8640 min Summer	1.420	0.0	850.3	5448	10080 min Summer	1.241	0.0	851.0	6256	15 min Winter	263.189	0.0	164.8	19
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status																																																																																																																																																																																																																																																																																	
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4, Ironstone Way Brixworth Northampton, NN3 9UD			Proposed AGP Northfleet School for Girls FEH, 1 in 100 yr + CC, 2.0 l/s				
Date 17/07/2025 10:25 File 28616 - PITCH CALCS.SRCX			Designed by TMK Checked by KER				
Innovyze			Source Control 2020.1.3				
<u>Summary of Results for 100 year Return Period (+45%)</u>							
Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
30 min Winter	30.019	0.366	0.0	2.0	2.0	361.8	Flood Risk
60 min Winter	30.041	0.388	0.0	2.0	2.0	402.5	Flood Risk
120 min Winter	30.063	0.410	0.0	2.0	2.0	444.6	FLOOD
180 min Winter	30.072	0.419	0.0	2.0	2.0	469.1	FLOOD
240 min Winter	30.078	0.425	0.0	2.0	2.0	485.6	FLOOD
360 min Winter	30.086	0.433	0.0	2.0	2.0	506.7	FLOOD
480 min Winter	30.090	0.437	0.0	2.0	2.0	519.1	FLOOD
600 min Winter	30.093	0.440	0.0	2.0	2.0	526.6	FLOOD
720 min Winter	30.095	0.442	0.0	2.0	2.0	530.9	FLOOD
960 min Winter	30.107	0.454	0.0	2.0	2.0	560.6	FLOOD
1440 min Winter	30.121	0.468	0.0	2.0	2.0	595.3	FLOOD
2160 min Winter	30.129	0.476	0.0	2.1	2.1	614.9	FLOOD
2880 min Winter	30.129	0.476	0.0	2.1	2.1	614.8	FLOOD
4320 min Winter	30.098	0.445	0.0	2.0	2.0	537.6	FLOOD
5760 min Winter	30.074	0.421	0.0	2.0	2.0	476.2	FLOOD
7200 min Winter	30.051	0.398	0.0	2.0	2.0	419.7	Flood Risk
8640 min Winter	30.022	0.369	0.0	2.0	2.0	367.3	Flood Risk
10080 min Winter	29.994	0.341	0.0	1.9	1.9	318.9	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)			
30 min Winter	146.195	0.0	166.0	34			
60 min Winter	81.208	0.0	328.2	64			
120 min Winter	45.109	4.3	331.0	122			
180 min Winter	31.982	13.3	331.9	182			
240 min Winter	25.057	19.7	332.3	240			
360 min Winter	17.765	27.6	332.0	358			
480 min Winter	13.919	32.8	330.9	476			
600 min Winter	11.518	35.5	329.5	592			
720 min Winter	9.868	36.6	327.8	708			
960 min Winter	8.061	48.6	326.0	942			
1440 min Winter	6.061	62.8	320.2	1398			
2160 min Winter	4.557	71.1	647.7	2056			
2880 min Winter	3.723	71.0	640.0	2680			
4320 min Winter	2.609	39.4	607.2	3288			
5760 min Winter	2.027	16.2	951.1	4208			
7200 min Winter	1.666	0.0	963.2	5112			
8640 min Winter	1.420	0.0	969.8	5960			
10080 min Winter	1.241	0.0	973.7	6760			
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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 100 yr + CC, 2.0 l/s	
Date 17/07/2025 10:25 File 28616 - PITCH CALCS.SRCX	Designed by TMK Checked by KER	
Innovyze Source Control 2020.1.3		

Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location	562900 172050 TQ 62900 72050
C (1km)	-0.027
D1 (1km)	0.276
D2 (1km)	0.421
D3 (1km)	0.247
E (1km)	0.324
F (1km)	2.535
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+45

Time Area Diagram

Total Area (ha) 0.643

Time (mins)	Area
From:	To: (ha)
0	4 0.643

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4, Ironstone Way Brixworth Northampton, NN3 9UD	Proposed AGP Northfleet School for Girls FEH, 1 in 100 yr + CC, 2.0 l/s	
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Innovyze Source Control 2020.1.3		

Model Details

Storage is Online Cover Level (m) 30.058

Porous Car Park Structure

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	61.0
Membrane Percolation (mm/hr)	1000	Length (m)	97.0
Max Percolation (l/s)	1643.6	Slope (1:X)	300.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	29.653	Membrane Depth (m)	0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0064-2000-1200-2000
Design Head (m)	1.200
Design Flow (l/s)	2.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	64
Invert Level (m)	28.858
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	2.0
Flush-Flo™	0.282	1.8
Kick-Flo®	0.573	1.4
Mean Flow over Head Range	-	1.6

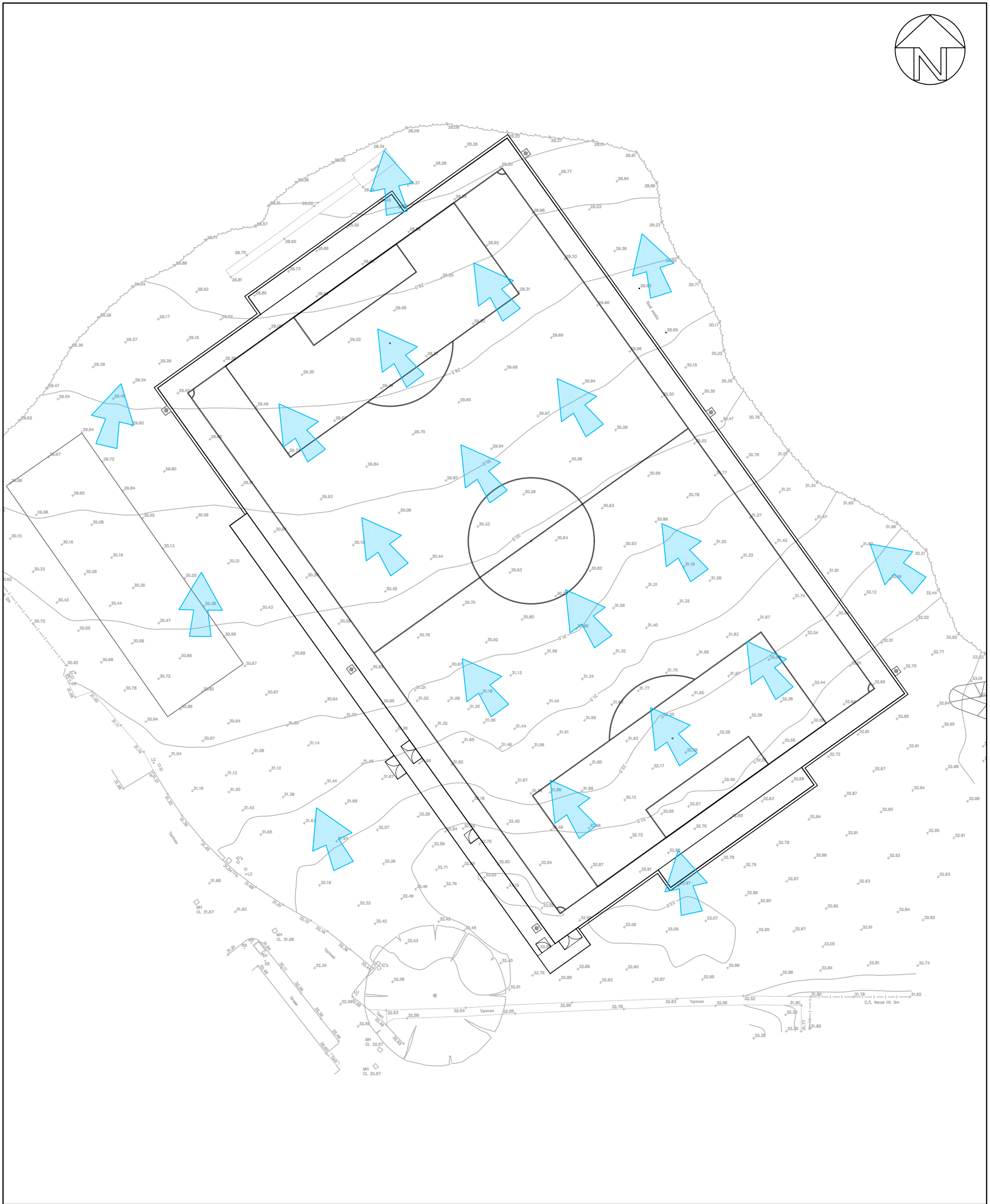
The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.5	1.200	2.0	3.000	3.0	7.000	4.5
0.200	1.7	1.400	2.1	3.500	3.3	7.500	4.7
0.300	1.8	1.600	2.3	4.000	3.5	8.000	4.8
0.400	1.7	1.800	2.4	4.500	3.7	8.500	5.0
0.500	1.6	2.000	2.5	5.000	3.9	9.000	5.1
0.600	1.5	2.200	2.6	5.500	4.0	9.500	5.2
0.800	1.7	2.400	2.7	6.000	4.2		
1.000	1.8	2.600	2.8	6.500	4.4		

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Appendix L
Overland Flows
JPP Consulting drawing no. 28616-FRA04



1. All dimensions are in metres unless otherwise stated.
2. All levels are in metres.
3. This drawing is to be read in conjunction with all relevant Engineers and Architect's drawings, Specifications, Reports and Engineering Details.
4. Do not scale from this drawing.

 Predicted Overland Flows

This drawing has been based/developed using the following external drawings					
Type	Company	Drawing date	Drawing title	Date received	Rev
Site Layout	Labosport Ltd	17.05.2024	Proposed Plan	11.07.2024	-
Topographical Survey	JPP Surveying	09.02.2024	Topographical Survey	11.07.2024	-

